1/19/2026 11:44:55 AM

SEISMIC UPGRADE

Beach Cities Health District
BCHD
A Public Agency

510 N. PROSPECT AVE., REDONDO BEACH, CA 90277

SELECTIVE DEMOLITION NOTES

1. OWNER WILL OCCUPY PORTIONS OF THE BUILDING IMMEDIATELY ADJACENT TO THE SELECTIVE DEMOLITION AREA. CONDUCT SELECTIVE DEMOLITION SO OWNER'S OPERATIONS WILL NOT BE DISRUPTED.
2. CONDITIONS EXISTING AT TIME OF INSPECTION FOR BIDDING PURPOSES WILL BE MAINTAINED BY OWNER AS FAR AS PRACTICAL.
3. NOTIFY THE ARCHITECT OF DISCREPANCIES BETWEEN EXISTING CONDITIONS AND DRAWINGS BEFORE PROCEEDING WITH SELECTIVE DEMOLITION.
4. HAZARDOUS MATERIALS: IT IS NOT EXPECTED THAT HAZARDOUS MATERIALS WILL BE ENCOUNTERED IN THE WORK. HAZARDOUS MATERIALS WILL BE REMOVED BY OWNER BEFORE THE START OF THE WORK. IF SUSPECTED HAZARDOUS MATERIALS ARE ENCOUNTERED, DO NOT DISTURB; IMMEDIATELY NOTIFY THE ARCHITECT AND OWNER. HAZARDOUS MATERIALS WILL BE REMOVED BY OWNER UNDER A SEPARATE CONTRACT.
5. STORAGE OR SALE OF REMOVED ITEMS OR MATERIALS ON-SITE IS NOT PERMITTED.
6. MAINTAIN EXISTING UTILITIES INDICATED TO REMAIN IN SERVICE AND PROTECT THEM AGAINST DAMAGE DURING SELECTIVE DEMOLITION OPERATIONS.
7. VERIFY THAT UTILITIES HAVE BEEN DISCONNECTED AND CAPPED BEFORE STARTING SELECTIVE DEMOLITION OPERATIONS.
8. REVIEW PROJECT RECORD DOCUMENTS OF EXISTING CONSTRUCTION OR OTHER EXISTING CONDITIONS AND HAZARDOUS MATERIAL INFORMATION PROVIDED BY THE OWNER. OWNER DOES NOT GUARANTEE THAT EXISTING CONDITIONS ARE THE SAME AS THOSE INDICATED IN PROJECT RECORD DOCUMENTS.
9. VERIFY THAT HAZARDOUS MATERIALS HAVE BEEN REMEDIATED BEFORE PROCEEDING WITH BUILDING DEMOLITION OPERATIONS.
10. TEMPORARY PROTECTION: PROVIDE TEMPORARY BARRICADES AND OTHER PROTECTION REQUIRED TO PREVENT INJURY TO PEOPLE AND DAMAGE TO ADJACENT BUILDINGS AND FACILITIES TO REMAIN.
11. PROVIDE PROTECTION TO ENSURE SAFE PASSAGE OF PEOPLE AROUND SELECTIVE DEMOLITION AREAS AND TO AND FROM OCCUPIED PORTIONS OF THE BUILDING.
12. PROVIDE TEMPORARY WEATHER PROTECTION, DURING THE INTERVAL BETWEEN SELECTIVE DEMOLITION OF EXISTING CONSTRUCTION ON EXTERIOR SURFACES AND NEW CONSTRUCTION, TO PREVENT WATER LEAKAGE AND DAMAGE TO STRUCTURE AND INTERIOR AREAS.
13. PROTECT WALLS, CEILINGS, FLOORS, AND OTHER EXISTING FINISH WORK THAT ARE TO REMAIN OR THAT ARE EXPOSED DURING SELECTIVE DEMOLITION OPERATIONS.
14. COVER AND PROTECT FURNITURE, FURNISHINGS, AND EQUIPMENT THAT HAVE NOT BEEN REMOVED.
15. COMPLY WITH REQUIREMENTS FOR TEMPORARY ENCLOSURES, DUST CONTROL, HEATING, AND COOLING
16. REMOVE TEMPORARY BARRICADES AND PROTECTIONS WHERE HAZARDS NO LONGER EXIST.
17. ERECT AND MAINTAIN DUSTPROOF AND FIREPROOF (WHEN APPLICABLE) PARTITIONS AS REQUIRED TO PREVENT SPREAD OF DUST, FUMES AND SMOKE TO OTHER PARTS OF THE BUILDING. ON COMPLETION, REMOVE PARTITIONS AND REPAIR DAMAGED SURFACES TO MATCH ADJACENT SURFACES.
18. LOCATE GUARDRAILS IN STAIRWELLS AND AROUND OPEN SHAFTS TO PROTECT WORKERS. POST CLEARLY VISIBLE WARNING SIGNS.
19. REMOVE TEMPORARY BARRICADES AND PROTECTIONS WHERE HAZARDS NO LONGER EXIST.
20. DEMOLISH AND REMOVE EXISTING CONSTRUCTION ONLY TO THE EXTENT REQUIRED BY NEW CONSTRUCTION AND AS INDICATED. USE METHODS REQUIRED TO COMPLETE THE WORK WITHIN LIMITATIONS OF GOVERNING REGULATIONS AND AS FOLLOWS:
    - A. PROCEED WITH SELECTIVE DEMOLITION SYSTEMATICALLY, FROM HIGHER TO LOWER LEVEL. COMPLETE SELECTIVE DEMOLITION OPERATIONS ABOVE EACH FLOOR OR TIER BEFORE DISTURBING SUPPORTING MEMBERS ON THE NEXT LOWER LEVEL.
    - B. NEATLY CUT OPENINGS AND HOLES PLUMB, SQUARE, AND TRUE TO DIMENSIONS REQUIRED. USE CUTTING METHODS LEAST LIKELY TO DAMAGE CONSTRUCTION TO REMAIN OR ADJOINING CONSTRUCTION. USE HAND TOOLS OR SMALL POWER TOOLS DESIGNED FOR SAWING OR GRINDING, NOT HAMMERING AND CHOPPING. TEMPORARILY COVER OPENINGS TO REMAIN.
    - C. CUT OR DRILL FROM THE EXPOSED OR FINISHED SIDE INTO CONCEALED SURFACES TO AVOID MARRING EXISTING FINISHED SURFACES.
    - D. DO NOT USE CUTTING TORCHES UNTIL THE WORK AREA IS CLEARED OF FLAMMABLE MATERIALS. AT CONCEALED SPACES, SUCH AS DUCT AND PIPE INTERIORS, VERIFY CONDITION AND CONTENTS OF HIDDEN SPACE BEFORE STARTING FLAME-CUTTING OPERATIONS. MAINTAIN PORTABLE FIRE-SUPPRESSION DEVICES DURING FLAME-CUTTING OPERATIONS.
    - E. MAINTAIN FIRE WATCH DURING AND FOR AT LEAST <INSERT NUMBER> HOURS AFTER FLAME-CUTTING OPERATIONS.
    - F. MAINTAIN ADEQUATE VENTILATION WHEN USING CUTTING TORCHES.
    - G. REMOVE DECAYED, VERMIN-INFESTED, OR OTHERWISE DANGEROUS OR UNSUITABLE MATERIALS AND PROMPTLY DISPOSE OF OFF-SITE.
21. EXISTING BUILT-UP ROOFING: REMOVE NO MORE EXISTING ROOFING THAN WHAT CAN BE COVERED IN ONE DAY BY NEW ROOFING AND SO THAT THE BUILDING INTERIOR REMAINS WATERTIGHT AND WEATHERTIGHT. PATCH AND REPAIR DAMAGED ROOFING CAUSED BY THE SCOPE OF WORK TO MATCH EXISTING ADJACENT ROOFING USING INDUSTRIAL STANDARD METHODS FOR THE TYPE OF ROOFING.
22. REMOVE, REPLACE, PATCH, AND REPAIR MATERIALS AND SURFACES CUT OR DAMAGED DURING SELECTIVE DEMOLITION, BY METHODS AND WITH MATERIALS AND USING APPROVED CONTRACTORS SO AS NOT TO VOID EXISTING WARRANTIES. NOTIFY THE WARRANTOR BEFORE PROCEEDING.

LANDLORD NOTES

1. THERE SHALL BE NO DISRUPTION OF THE BUILDING UTILITIES TO EXISTING TENANTS. 48 HOUR PRIOR TO NOTICE AND SCHEDULING WITH LANDLORD IS REQUIRED.
2. ALL ACCESS PANEL LOCATIONS SHALL BE APPROVED BY CHIEF ENGINEER. GENERAL CONTRACTOR AND MEP ENGINEERS TO COORDINATE.
3. THERE SHALL BE QUIET ENJOYMENT IN THE BUILDING FOR ALL EXISTING TENANTS.
4. FIRE LIFE SAFETY SYSTEMS AND EQUIPMENT SHALL REMAIN OPERABLE AT ALL TIMES. IF OPERATION OF SAME IS OR WILL BE COMPROMISED AT ANY TIME DURING COURSE OF THIS PROJECT, APPROPRIATE FIRE WATCH SHALL BE POSTED AT CONTRACTOR'S EXPENSE.
5. ANY SCHEDULED SPRINKLER OR ALARM WORK REQUIRES SUBMISSION OF AN IMPAIRMENT FORM TO THE PROJECT MANAGEMENT OFFICE FOR APPROVAL 2 BUSINESS DAYS IN ADVANCE OF PROPOSED WORK. IN THE EVENT OF ACCIDENTAL DISCONNECT OR DAMAGE TO ANY FLS SYSTEM, TS ENGINEERING SHALL BE NOTIFIED IMMEDIATELY.
6. ALL WORK TO BE PERFORMED SHALL BE PERMITTED INSPECTED AND APPROVED BY THE REDONDO BEACH BUILDING AND SAFETY AND FIRE DEPARTMENT. COPIES OF PERMIT AND SIGNED PERMIT CARDS AND SUBMITTED TITLE 24 & GREEN CODE FORMS TO BE SUBMITTED AS PART OF THE CLOSEOUT PACKAGE.
7. VERIFY WITH LANDLORD PRIOR TO ALL MEP DECK PENETRATIONS.
8. COORDINATE ACCESS TO BUILDING AND LARGE DELIVERIES WITH PROJECT MANAGEMENT OFFICES.
9. ANY WORK REQUIRING ACCESS TO OTHER TENANT AREAS OR DISRUPTIONS TO BASE BUILDING SERVICES REQUIRES ADVANCE COORDINATION WITH PROJECT MANAGEMENT OFFICE.
10. CONTRACTOR AND TENANT REPRESENTATIVE TO REVIEW AND SIGN CONSTRUCTION RULES AND REGULATIONS AND RETURN TO PROJECT MANAGEMENT OFFICE. PROJECT DIRECTORY AND INSURANCE CERTIFICATIONS TO BE PROVIDED PRIOR TO COMMENCEMENT OF ANY WORK.
11. INTERIOR SUITE SIGNAGE SHALL BE TENANT'S RESPONSIBILITY.

SHEET INDEX

A001	EXISTING SITE PLAN
G001	PROJECT INFO, SHEET INDEX
S001	GENERAL NOTES
S002	GENERAL NOTES
S101	UPPER FRAMING PLAN
S102	ROOF FRAMING PLAN
S201	DETAIL

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APPLICABLE CODES

2025 CALIFORNIA ADMINSTRATIVE CODE (C.A.C.), PART 1, TITLE 24 C.C.R.

2025 CALIFORNIA BUILDING CODE (CBC) - PART 2 TITLE 24, CCR

2025 CALIFORNIA ELECTRICAL CODE (CEC) - PART 3 TITLE 24, CCR

2025 CALIFORNIA MECHANICAL CODE (CMC) - PART 4 TITLE 24, CCR

2025 CALIFORNIA PLUMBING CODE (CPC) - PART 5 TITLE 24, CCR

2025 CALIFORNIA ENERGY CODE - PART 6 TITLE 24, CCR

2025 CALIFORNIA FIRE CODE (CFC) - PART 9 TITLE 24, CCR

2025 CALIFORNIA GREEN BUILDING STANDARDS CODE - PART 11 TITLE 24, CCR

2025 CALIFORNIA REFERENCE STANDARDS CODE - PART 12 TITLE 24, CCR

PROJECT INFORMATION

PROJECT ADDRESS:	510 NORTH PROSPECT AVE, REDONDO BEACH, CA 90277
DESCRIPTION:	VOLUNTARY SEISMIC STRENGTHENING OF AN EXISTING 3-STORY MEDICAL OFFICE BUILDING WITH CMU SHEAR WALLS. STRENGTHENING IS LIMITED TO NEW STEEL COLLECTORS AT THE UPPER LEVEL AND ROOF.
CONSTRUCTION TYPE:	V OFFICE BLDG SPRINKLER SYSTEM THROUGHOUT
NUMBER OF STORIES:	2 WITH SUBTERRANEAN LEVEL
PROJECT SCOPE:	13,984 SF (15,403 SF GROSS)
EXISTING OCCUPANCY:	OCCUPANCY GROUP B-2
ZONING	P-CF

LEGAL INFORMATION

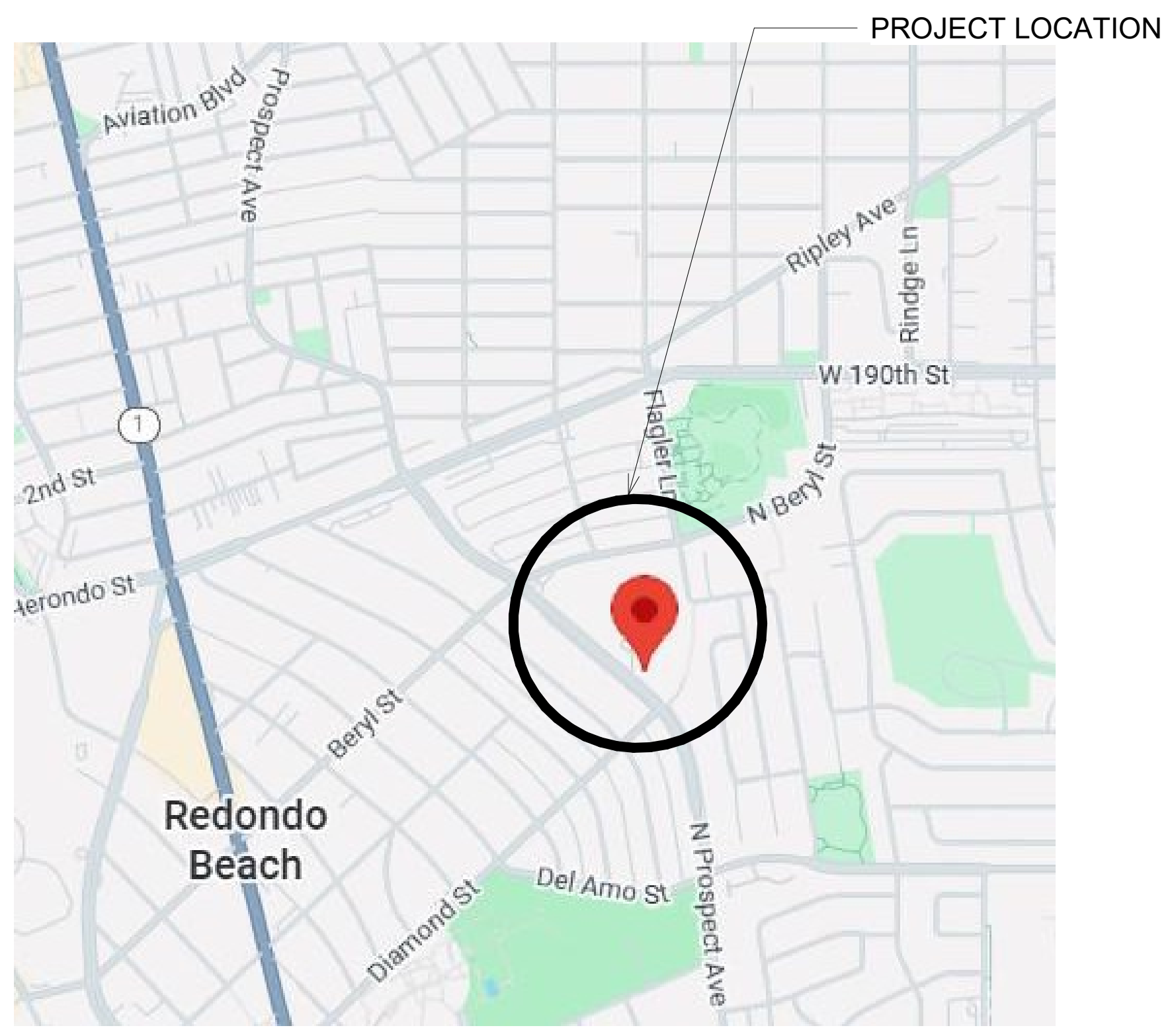
PIN NUMBER	-
LOT/PARCEL AREA	9.94 AC
ASSESSOR PARCEL NO. (APN)	7502-017-904
LAND USE CODE	INSTITUTIONAL

BUILDING CODE ANALYSIS

TABLE 601

FIRE RESISTANCE RATING FOR BUILDING ELEMENTS	TYPE VB
ROOF CONSTRUCTION & ASSOCIATED MEMBERS	0
PRIMARY STRUCTURAL FRAME	0
FLOOR CONSTRUCTION AND ASSOCIATED SECONDARY MEMBERS	0

VICINITY MAP



STRUERE
ADVANCED ARCHITECTURE

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28713

Beach Cities Health District
BCHD
A Public Agency

SEISMIC UPGRADE

510 N PROSPECT AVE
REDONDO BEACH, CA 90277

BCHD - CENTER FOR HEALTH & FITNESS

ISSUE DESCRIPTION

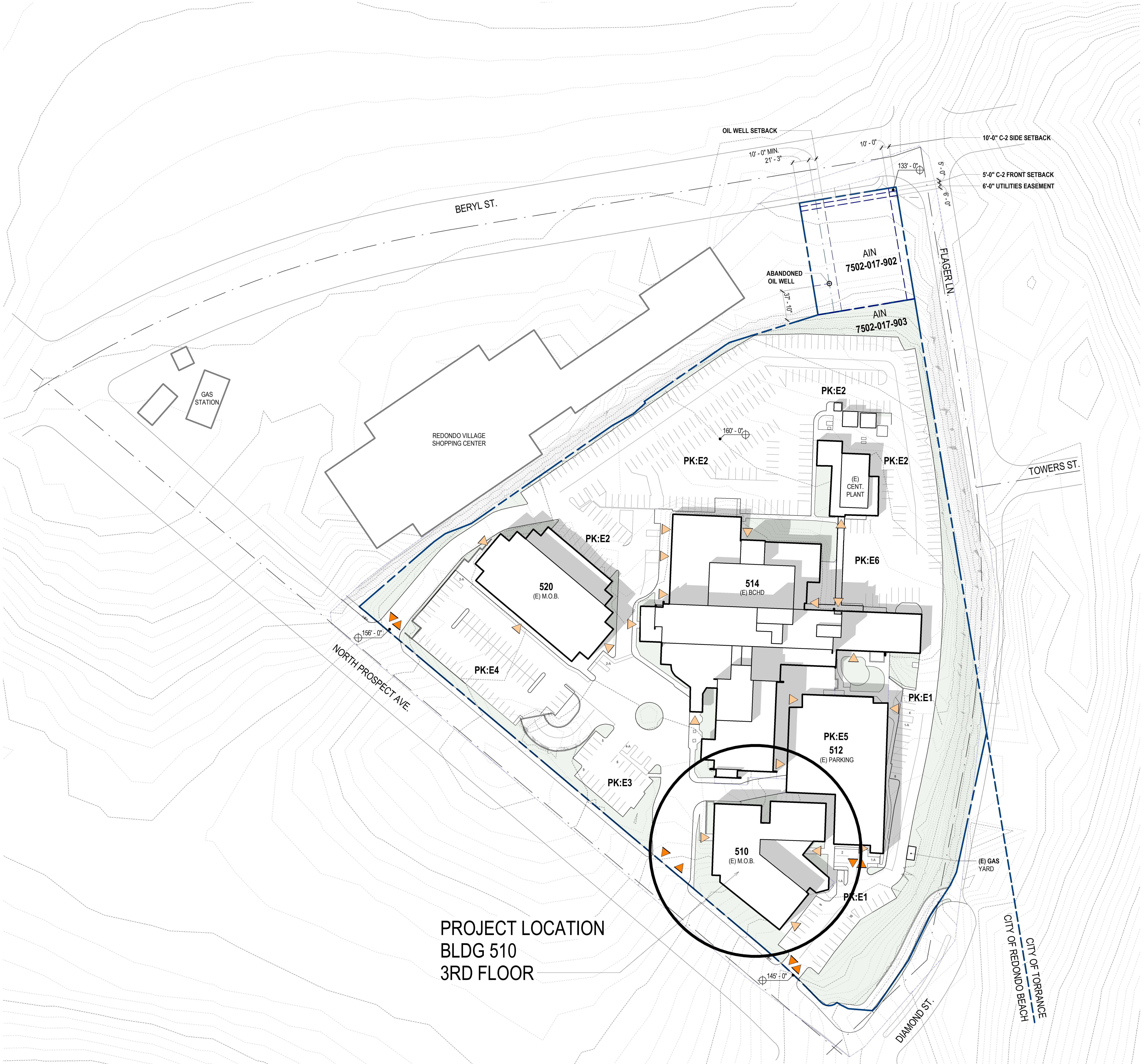
2	ISSUE FOR PLAN CHECK	01.19.2026

SHEET TITLE

PROJECT INFO, SHEET INDEX

SHEET NUMBER

G001



EXISTING - SITE PLAN ①
1" = 50'-0"

EXISTING LAND USE DESIGNATIONS AND ZONING

AIN:	**7502-017-903**
ZONING DISTRICT	**P-CF COMMUNITY FACILITY ZONE**
LAND USE:	**P PUBLIC OR INSTITUTIONAL**
AREA:	**9.94 ACRES**
AIN:	**750-2017-902**
ZONING DISTRICT	**C-2 COMMERCIAL ZONE**
LAND USE:	**C-2, COMMERCIAL**
AREA:	**0.43 ACRES**

LAND USE DESIGNATIONS AND ZONING REFERENCES:

- CITY OF REDONDO BEACH GENERAL PLAN (CITY OF REDONDO BEACH 2008, 2011, SECTION 3.10 LAND USE AND PLANNING),
- REDONDO BEACH ZONING ORDINANCE
- REDONDO BEACH MUNICIPAL CODE TITLE 10 PLANNING AND ZONING CHAPTER 2 ZONING AND LAND USE

EXISTING SITE OPEN SPACE

HILLSIDE	50,950 SF
OTHER	31,910 SF
TOTAL EXISTING OPEN SPACE	**82,940 SF**

EXISTING PARKING

PK:E1	**41** SPACES	**3** ACCESSIBLE
PK:E2	**257** SPACES	**3** ACCESSIBLE
PK:E3	**24** SPACES	**4** ACCESSIBLE
PK:E4*	**281** SPACES	**8** ACCESSIBLE
PK:E5*	**199** SPACES	**2** ACCESSIBLE
PK:E6*	**11** SPACES	**0** ACCESSIBLE
TOTAL*	**813 TOTAL**	**20 ACCESSIBLE**

***TOTAL PK:E4**	**281 SPACES**	**8 ACCESSIBLE**
SURFACE	62 SPACES	4 ACCESSIBLE
P1	107 SPACES	4 ACCESSIBLE
P2	112 SPACES	0 ACCESSIBLE
***TOTAL PK:E5**	**199 SPACES**	**2 ACCESSIBLE**
TOP DECK	59 SPACES	0 ACCESSIBLE
2ND LEVEL	52 SPACES	0 ACCESSIBLE
1ST LEVEL	53 SPACES	2 ACCESSIBLE
UNDERGROUND	35 SPACES	0 ACCESSIBLE
***TOTAL (E)** SURFACE	395 SPACES	14 ACCESSIBLE
***TOTAL (E)** STRUCTURE	418 SPACES	6 ACCESSIBLE

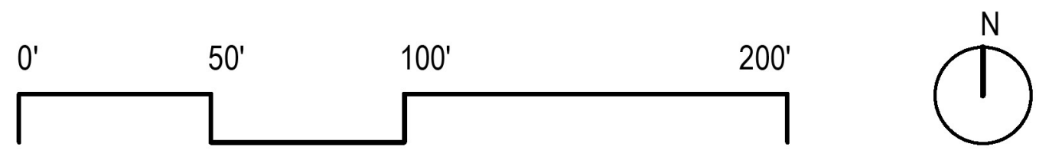
*PARKING SPACES SUBTOTALS + TOTALS INCLUDE ACESSIBLE SPACES.

SYMBOL LEGEND

◁ PEDESTRIAN ENTRY / EXIT

◀ VEHICULAR ENTRY / EXIT

(REFERENCE ONLY)



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NO.	DATE	REVISION

BEACH CITIES
HEALTH DISTRICT
HEALTHY LIVING
CAMPUS

514 NORTH PROSPECT AVE.
REDONDO BEACH, CA 90277

PMA PROJECT NO.
19010

DRAWING TITLE
EXISTING SITE PLAN

SCALE
AS INDICATED

DATE
02/02/2022

DRAWN	CHECKED
EC	PM

SHEET NO.
(E) A100
(REFERENCE ONLY)

1/14/2026 3:03:12 PM

STRUCTURAL STEEL:

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, AND THE LATEST EDITION OF AISC SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS. WHERE THE STRUCTURAL STEEL IS EXPOSED, FABRICATION AND ERECTION SHALL ALSO BE IN ACCORDANCE WITH AISC CODE OF STANDARD PRACTICE FOR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL.

2. STRUCTURAL STEEL SHALL CONFORM TO ASTM DESIGNATION AS INDICATED BELOW (U.N.O.):

ALL WIDE FLANGE SHAPES	A992 OR A572, GR. 50
STEEL ANGLES	A36
ALL OTHER STRUCTURAL SECTIONS	A572, GR. 50
ALL PLATES	A36, U.N.O.
STRUCTURAL TUBING	A500, GR. C
STRUCTURAL PIPING	A53, GR. B
STAINLESS STEEL SHAPES, PLATES AND BARS	A276
BOLTS	A325-X
ANCHOR BOLTS	F1554, GR 36
THREADED RODS	A36
NUTS FOR BOLTS AND MACHINE BOLTS	A563
HARDENED WASHERS	F436
UNHARDENED WASHERS	F844
PLAIN WASHERS	ANSI B18.22.1
BEVELED WASHERS	ANSI B18.23.1

3. HEAVY STRUCTURAL SECTIONS INCLUDE, ASTM A6 GROUP 3 SHAPES WITH FLANGES 2" THICK OR THICKER, ASTM A6 GROUP 4 AND 5 SHAPES, AND PLATES THAT ARE 2" THICK OR THICKER IN BUILT-UP CROSS-SECTIONS.

4. HEAVY STRUCTURAL SECTIONS SHALL BE SUPPLIED WITH TESTING IN ACCORDANCE WITH ASTM A6 SUPPLEMENTARY REQUIREMENT S5 & SHALL HAVE A MINIMUM CHARPY V-NOTCH (CVN) TOUGHNESS OF 20 FT/LBS AT 70 DEGREES FAHRENHEIT.

5. WHEN FABRICATING BEAMS PLACE NATURAL CAMBER UP.

6. SPLICE MEMBERS ONLY WHERE INDICATED OR APPROVED BY THE OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER.

7. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF AISC SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. HIGH STRENGTH BOLTS SHALL BE BEARING TYPE WITH THREADS EXCLUDED FROM THE FROM THE SHEAR PLANES (I.E. A325-X) U.N.O..

8. ALL BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS UNLESS SHOWN OTHERWISE. MINIMUM SIZE OF BOLTS FOR STRUCTURAL STEEL CONNECTIONS SHALL BE 7/8" DIA. EXCEPT WHEN OTHERWISE SHOWN OR NOTED.

9. ALL HOLES SHALL BE STANDARD DIAMETER U.N.O.

10. PROVIDE HARDENED WASHERS UNDER NUTS AT ALL HIGH STRENGTH BOLTS, EXCEPT WHERE PLATE WASHERS ARE USED PER AISC SPECIFICATIONS.

11. ALL FLANGE STIFFENER PLATES SHALL BE ORIENTED SO THAT ROLLING DIRECTION OF PLATE IS PARALLEL WITH DIRECTION OF PRINCIPAL STRESS.

12. AFTER FABRICATION, ALL STEEL SHALL BE CLEANED FREE OF RUST, LOOSE MILL SCALE AND OIL.

13. PROVIDE FILLS AT SPLICES OF PARTS HAVING MORE THAN 1/8" DIFFERENCE IN THICKNESS.

14. PROVIDE BEVELED WASHERS ON ALL CONNECTIONS WHERE SLOPE SURFACE EXCEEDS 1:20.

15. HEADED ANCHOR STUDS AND THREADED STUDS SHALL BE NELSON FLUXED HEADED ANCHOR STUDS (ICC ESR-2856 / LARR #02725) CONFORMING TO ASTM A108. STUDS SHALL BE AUTOMATICALLY END WELDED IN THE FIELD. STUD WELDING INSPECTION AND TESTING SHALL CONFORM TO AWS D1.1 AND THE MANUFACTURER'S RECOMMENDATIONS.

16. PROVIDE 3/4"ø HEADED SHEAR STUDS AT 12" O.C. ON ALL STEEL BEAMS SUPPORTING (N) CONCRETE SLABS, U.N.O.

17. DEFORMED BAR ANCHORS SHALL BE NELSON D2L DEFORMED BAR ANCHORS (ICC ESR-2907 / LARR #25860) CONFORMING TO ASTM A496. ANCHOR WELDING INSPECTION AND TESTING SHALL CONFORM TO AWS D1.1 AND THE MANUFACTURER'S RECOMMENDATIONS.

18. DEFORMED BAR LENGTH EQUAL TO D2L DEVELOPMENT LENGTH IN TENSION, U.N.O.

19. HOT DIP GALVANIZE IN ACCORDANCE WITH ASTM A123 AND ASTM A153 STRUCTURAL STEEL AND FASTENERS THAT ARE PERMANENTLY EXPOSED TO THE WEATHER. REPAIR GALVANIZING AFTER WELDING IN ACCORDANCE WITH ASTM A780.

20. STRESSES OCCURRING DURING FABRICATION, SHIPMENT, AND ERECTION SHALL BE TEMPORARY AND NOT EXCESSIVE. STRESSES AT ALL TIMES SHALL BE LESS THAN DESIGN AND ALLOWABLE STRESSES. THE FULL DESIGN AND LOAD CARRYING CAPACITY OF THE STEEL WORK SHALL NOT BE IMPAIRED DUE TO FABRICATION, SHIPMENT, OR ERECTION PROCEDURES, THROUGHOUT THE COMPLETE PROCESS. THE STABILITY OF ALL INDIVIDUAL MEMBERS AND ASSEMBLIES SHALL BE MAINTAINED.

21. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIALS AND WELD SHRINKAGE.

22. ALL ADDITIONAL STEEL REQUIRED FOR ERECTION PURPOSES SHALL BE PROVIDED AT NO ADDITIONAL COST AND SHALL BE REMOVED UNLESS APPROVED BY THE OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER.

23. SEE ARCHITECTURAL PLANS FOR DETAILS OF FIREPROOFING AND GOVERNING JURISDICTION REQUIREMENTS.

24. SHORING ANCHOR RODS SHALL BE DEFORMED WITH ULTIMATE TENSILE STRENGTH OF 150 KSI CONFORMING TO ASTM A722 GR. 150 SPECIFICATIONS. USE DYWIDAG TIEBACK ANCHORS (LARR #23835) OR EQUAL.

STRUCTURAL STEEL WELDING:

1. ALL WELDING SHALL BE IN STRICT CONFORMANCE WITH THE LATEST EDITION OF AWS D1.1 AND THE CALIFORNIA BUILDING CODE WITH ALL APPLICABLE AMENDMENTS.

2. ALL WELDING ELECTRODES (FILLER METAL) SHALL BE E7XXX (70 KSI), U.N.O., AND SHALL BE LOW HYDROGEN TYPES. E8XXX (80 KSI) SHALL BE USED AT COLUMN SPLICES FOR A913 GRADE 65 STEEL.

3. ALL WELDS SHALL HAVE A FILLER METAL WITH CHARPY V-NOTCH TOUGHNESS OF 20 FT/LBS AVERAGE AT -20 DEGREES FAHRENHEIT AND 40 FT/LBS @ 70 DEGREES FAHRENHEIT. CERTIFY CONFORMANCE TO CHARPY V-NOTCH TOUGHNESS REQUIREMENTS WITH TESTS BY AN INDEPENDENT TESTING LABORATORY.

4. LENGTHS OF WELDS ARE EFFECTIVE LENGTHS AS SPECIFIED IN THE CALIFORNIA BUILDING CODE. WHERE LENGTH OF WELD IS NOT SHOWN IT SHALL BE FULL LENGTH OF JOINT. ALL BUTT WELDS SHALL BE FULL PENETRATION, U.N.O..

5. WHERE MINIMUM AISC FILLET WELD THICKNESS REQUIREMENT EXCEEDS WELDS SHOWN ON DETAILS, PROVIDE MINIMUM AISC WELD.

6. ALL SHOP WELDS SHALL BE PERFORMED BY A LADBS LICENSED FABRICATOR'S SHOP. FIELD WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED BY LADBS FOR STRUCTURAL STEEL.

7. CONTRACTOR SHALL PROVIDE FIELD WELDING AS REQUIRED FOR CONSTRUCTION. WHERE FIELD WELDING IS NOTED, THE DESIGNATION IS GIVEN AS A SUGGESTED CONSTRUCTION PROCEDURE ONLY.

8. ALL WELDERS SHALL BE QUALIFIED FOR THE WORK THEY WILL BE DOING & SHALL HAVE CERTIFICATIONS CURRENT AND CERTIFIED BY AWS & THE GOVERNING JURISDICTION.

9. FACES OF FILLET WELDS EXPOSED TO VIEW SHALL HAVE AS-WELDED SURFACES THAT ARE REASONABLY SMOOTH AND UNIFORM. NO FINISHING OR GRINDING SHALL BE REQUIRED, EXCEPT WHERE CLEARANCES OR FIT OF OTHER ITEMS MAY SO NECESSITATE.

10. ALL PARTIAL AND FULL PENETRATION WELDS WHICH ARE EXPOSED TO VIEW SHALL BE GROUND SMOOTH AND FLUSH WITH FINISH SURFACE OF STEEL. HOLES SHALL BE FILLED WITH WELD METAL OR BODY SOLDER AND SMOOTHED BY GRINDING OR FILING.

11. CLEAN GROOVE PREPARATION THERMAL CUTS BY GRINDING.

12. WELDS SHALL BE TERMINATED AT THE END OF A JOINT IN A MANNER THAT WILL ENSURE SOUND WELDS. WHENEVER NECESSARY THIS SHALL BE DONE BY USE OF EXTENSION BARS AND RUN OFF TABS.

13. ALL WELDED JOINTS SHALL BE PRE-QUALIFIED PER THE LATEST EDITION OF AWS D1.1. NON PRE-QUALIFIED WELDED JOINTS SHALL BE QUALIFIED BY TEST & PROCEDURE QUALIFICATION TEST RECORD INCLUDED PER THE LATEST EDITION OF AWS D1.1.

14. THE CONTRACTOR SHALL SUBMIT ALL WELDING PROCEDURE SPECIFICATIONS (WPS) TO BE USED ON THE PROJECT PER THE LATEST EDITION OF AWS D1.1. FOR ALL WPS THAT ARE NOT PRE-QUALIFIED PER AWS D1.1, THE SUPPORTING PROCEDURE QUALIFICATION RECORD (PQR) SHALL ALSO BE SUBMITTED WITH THE WPS. THE WPS SHALL INCLUDE ALL MANUFACTURER'S DATA SHEETS FOR ALL WELDING MATERIALS TO BE USED. THE DATA SHEETS SHALL DESCRIBE THE PRODUCTS, LIMITATIONS OF USE, RECOMMENDED WELDING PARAMETERS, AND STORAGE AND EXPOSURE REQUIREMENTS.

15. THE CONTRACTOR SHALL PROVIDE PLANS SHOWING THE SEQUENCE OF WELDING PLANNED TO MINIMIZE LOCKED IN STRESSES AND DISTORTION IN THE STEEL FRAMING.

16. ELECTRODES SHALL BE RECEIVED AND STORED IN THE ORIGINAL, UNDAMAGED MANUFACTURER PACKAGING, UNTIL READY FOR USE. WHEN WELDING IS TO BE SUSPENDED FOR MORE THAN 8 HOURS, ELECTRODES SHALL BE REMOVED FROM THE MACHINES AND STORED IN AN ELECTRODE WIRE OVEN MAINTAINED AT A TEMPERATURE BETWEEN 250 DEGREES AND 550 DEGREES OR AS RECOMMENDED BY THE MANUFACTURER. ELECTRODES NOT CONSUMED WITHIN 24 HOURS OF ACCUMULATED EXPOSURE OUTSIDE CLOSED OR HEATED STORAGE SHALL NOT BE USED.

17. ALL BACKING BARS SHALL BE REMOVED. FOLLOWING REMOVAL OF BACKING, THE ROOT PASS SHALL BE BACKGOUGED TO SOUND WELD METAL AND BACKWELDED UNTIL FLUSH OR WITH SLIGHT REINFORCEMENT. THE SURFACE SHALL BE GROUND SMOOTH TO A SURFACE ROUGHNESS NOT TO EXCEED 500 MICROINCHES.

18. MINIMUM PREHEAT AND INTERPASS TEMPERATURES SHALL BE PROVIDED FOR ALL WELDS, INCLUDING TACK WELDS, IN ACCORDANCE WITH AWS D1.1 THE MAXIMUM PREHEAT AND MAXIMUM INTERPASS TEMPERATURE PERMITTED IS 550 DEGREES FAHRENHEIT, MEASURED AT A DISTANCE OF 1" FROM THE POINT OF ARC INITIATION.

18. WELDS MADE TO EXISTING STRUCTURAL STEEL SHALL BE VALIDATED AND DOCUMENTED WITH WPS (PROCESS AND MATERIAL) TO BE SUITABLE FOR ATTACHMENT OF (N) STEEL TO (E) STEEL. REPRESENTATIVE CHEMICAL STRENGTH PROPERTIES OF (E) STEEL IS AVAILABLE FOR CONTRACTOR'S USE IN DEVELOPING WPS. A PQR MAY BE NEEDED TO VERIFY WELD PROCESSES. REFER TO STEEL PROPERTIES TABLE FOR CHEMICAL AND STRENGTH PROPERTIES OF (E) STEEL (SHEET XXX).

19. PJP GROOVE WELD SIZES SPECIFIED REFER TO THE EFFECTIVE THROAT SIZE REGARDLESS OF THE USE OF PARENTHESES. ADJUST WELD PROCESS AND POSITION AS REQUIRED TO ACCOMPLISH EFFECTIVE THROAT.

20. WHERE REQUIRED TO ACCOMPLISH THE SPECIFIED WELD SIZE, OR WHERE NO WELD SIZE IS NOTED, FLARE BEVEL AND FLARE V-GROOVE WELDS ARE TO USE GMAW OR FCAW-G WELDING PROCESSES TO ACCOMPLISH THE MAXIMUM EFFECTIVE THROATS PERMITTED BY AISC 360 TABLE J2.2. WHERE THESE WELD PROCESSES ARE NOT FEASIBLE TO APPLY, NOTIFY SEOR FOR DIRECTION.

STRUCTURAL TEST AND INSPECTION:

1. SPECIAL INSPECTION IS REQUIRED. SEE SHEET S-XXX FOR STATEMENT OF SPECIAL INSPECTION.

2. CONTRACTORS RESPONSIBLE FOR THE CONSTRUCTION OF A WIND OR SEISMIC FORCE RESISTING SYSTEM/COMPONENT LISTED IN THE "STATEMENT OF SPECIAL INSPECTIONS" SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE LADBS INSPECTORS AND THE OWNER PRIOR TO COMMENCEMENT OF THE WORK ON SUCH SYSTEM OF COMPONENT.

3. THE FOLLOWING ELEMENTS ARE PART OF THE LATERAL FORCE RESISTING SYSTEM (LFRS) OF THE BUILDING

A. A. COLLECTORS
B. B. DIAPHRAGM TO WALL CONNECTIONS

4. APPROVAL BY THE INSPECTOR DOES NOT MEAN APPROVAL OF FAILURE TO COMPLY WITH THE PLANS OR SPECIFICATIONS. ANY DETAIL THAT FAILS TO BE CLEAR OR IS AMBIGUOUS MUST BE REFERRED TO THE OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER FOR INTERPRETATION OR CLARIFICATION.

5. APART FROM VISUAL INSPECTION AND REVIEW OF FABRICATION AND ERECTION REPORTS OF THE FABRICATOR/ERECTOR'S OWN QUALITY CONTROL TESTING AND INSPECTION, THE OWNER'S STEEL TESTING AGENCY SHALL PERFORM THE INDICATED SHOP AND FIELD INSPECTION AND TESTING. THE STEEL TESTING AGENCY SHALL BE AWS Q.C.-1 CERTIFIED BY THE LOCAL BUILDING AUTHORITY AND SHALL PROVIDE REGISTERED INSPECTORS FOR CONTINUOUS INSPECTION OF ALL STEEL FABRICATION AND ERECTION, STRUCTURAL WELDING, HIGH STRENGTH BOLTING, ETC. SHOP AND FIELD TESTING OF MATERIALS. WELDS AND BOLTS SHALL BE AS FOLLOWS:

A. ALL WELDS SHALL BE VISUALLY INSPECTED AND PERIODICALLY MEASURED (15% MIN.)

B. CJP WELDS: ULTRASONIC TESTING IS REQUIRED FOR ALL (100%) COMPLETE PENETRATION WELDS. TESTING SHALL BE PERFORMED 24 HOURS OR MORE AFTER COMPLETION OF WELDING, WELD BACKING REMOVAL AREAS AND FILLET WELDS SHALL BE SUBJECTED TO MAGNETIC PARTICLE EXAMINATION.

C. PJP WELDS: ULTRASONIC TESTING AND MAGNETIC PARTICLE EXAMINATION IS REQUIRED FOR HALF (50%) OF PARTIAL PENETRATION WELDS TO VERIFY DEPTH OF PJP WELD. TESTING SHALL BE PERFORMED 24 HOURS OR MORE AFTER COMPLETION OF WELDING.

D. FILLET WELDS: FOR FILLET WELDS GREATER THAN 5/16", CHECK A MINIMUM OF 10% OF WELDS BY MAGNETIC PARTICLE METHOD (ASTM 109). FOR CONTINUITY PLATE AND BEAMS WEB FILLET WELDS OF ANY SIZE, CHECK A MINIMUM OF 25% FOR BEAM FLANGE FILLET WELDS, CHECK 100%.

G. WELD ELECTRODES: AMPERAGE, VOLTAGE, POLARITY AND ELECTRODE STICK OUT SHALL BE BE VERIFIED TO BE IN COMPLIANCE WITH THE ELECTRODE MANUFACTURER' RECOMMENDATIONS.

H. WELD BASE METAL: BASE METAL THICKER THAN 1 1/2 INCHES, SUBJECTED TO THROUGH THICKNESS WELD SHRINKAGE, SHALL NE ULTRASONICALLY TESTED DIRECTLY BEHIND SUCH WELDS 48 HOURS OR MORE AFTER COMPLETION OF WELDING.

I. BOLTS: TEST BY CALIBRATED TORQUE WRENCH A MINIMUM OF ONE (1) HIGH STRENGTH BOLT AT EACH SHEAR CONNECTION. A MINIMUM OF TWO (2) BOLTS PER CONNECTION FOR CONNECTION WITH 8 BOLTS OR MORE AND A MINIMUM OF 2 BOLTS PER FRAME BEAM SHEAR CONNECTION.

5. SPECIAL INSPECTIONS OF THE FABRICATION PROCESS OF PREFABRICATED WOOD STRUCTURAL ELEMENTS SHALL BE IN ACCORDANCE WITH CBC SECTION 1704.2.

6. WOOD INSPECTION REQUIREMENTS SHALL BE PER CBC SECTION 1704.6.1.

7. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS, INCLUDING NAILING, BOLTING, ANCHORING AND OTHER FASTENING TO COMPONENTS OF THE SEISMIC FORCE RESISTING SYSTEMS. SPECIAL INSPECTION BY A DEPUTY INSPECTOR IS REQUIRED WHERE THE FASTENER SPACING OF THE SHEATHING IS 4" ON CENTER OR LESS.

STRUCTURAL OBSERVATIONS:

1. PERIODIC STRUCTURAL OBSERVATION PER CBC SECTION 1702 SHALL BE PROVIDED BY NABIH YOUSSEF ASSOCIATES.

2. STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE ELEMENTS AND CONNECTIONS OF THE STRUCTURAL SYSTEM AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS.

3. STRUCTURAL OBSERVATIONS PERFORMED BY ENGINEER DURING CONSTRUCTION ARE NOT CONTINUOUS AND SPECIAL INSPECTION SERVICES AND DO NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED OF THE BUILDING INSPECTOR OR THE DEPUTY INSPECTOR. OBSERVATIONS ALSO DO NOT GUARANTEE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSIDERED AS SUPERVISION OF CONSTRUCTION.

4. PRIOR TO THE FIRST STRUCTURAL OBSERVATION, THE OWNER OR OWNER'S REPRESENTATIVE SHALL COORDINATE AND CALL FOR A PRE-CONSTRUCTION MEETING BETWEEN THE ENGINEER, CONTRACTOR, AFFECTED SUBCONTRACTORS AND DEPUTY INSPECTORS. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY THE MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT AFFECT THE VERTICAL AND LATERAL LOAD SYSTEMS OF THE STRUCTURE AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS.

5. THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER:

CONSTRUCTION STAGES	ELEMENTS/CONSTRUCTION TO BE OBSERVED
STRUCTURAL STEEL	COLLECTORS, DIAPHRAGM TO WALL CONNECTIONS

6. NOTIFY ENGINEER AT LEAST 48 HOURS IN ADVANCE OF THE ABOVE CRITICAL STAGES OF CONSTRUCTION SO THAT THE ENGINEER MAY SCHEDULE VISITS FOR STRUCTURAL OBSERVATIONS

7. THE STRUCTURAL OBSERVER SHALL PREPARE A REPORT FOR EACH SIGNIFICANT STAGE OF CONSTRUCTION OBSERVED. THE ORIGINAL OF THE OBSERVATION REPORT SHALL BE SENT TO THE OWNER AND SHALL BE SIGNED AND SEALED (WET STAMP) BY THE RESPONSIBLE STRUCTURAL OBSERVER.

8. A FINAL OBSERVATION REPORT MUST BE SUBMITTED WHICH SHOWS THAT ALL OBSERVED DEFICIENCIES WERE RESOLVED AND THE STRUCTURAL SYSTEM GENERALLY CONFORMS WITH THE APPROVED PLANS AND SPECIFICATIONS. THE DEPARTMENT OF BUILDING AND SAFETY WILL NOT ACCEPT THE STRUCTURAL WORK WITHOUT THIS FINAL OBSERVATION REPORT AND THE CORRECTION OF SPECIFIC DEFICIENCIES NOTED DURING NORMAL BUILDING AND DEPUTY INSPECTION.

9. WHEN THE OWNER ELECTS TO CHANGE THE STRUCTURAL OBSERVER OF RECORD, THE OWNER SHALL:

A. NOTIFY THE BUILDING INSPECTOR IN WRITING BEFORE THE NEXT INSPECTION.
B. CALL AN ADDITIONAL PRE-CONSTRUCTION MEETING.
C. FURNISH THE REPLACEMENT STRUCTURAL OBSERVER WITH A COPY OF ALL PREVIOUS OBSERVATIONS REPORTS.

10. THE REPLACEMENT STRUCTURAL OBSERVER SHALL APPROVE THE CORRECTION OF THE ORIGINAL OBSERVED DEFICIENCIES UNLESS OTHERWISE APPROVED BY PLAN CHECK SUPERVISION.

GENERAL NOTES:

1. TYPICAL DETAILS AND GENERAL NOTES APPLY TO ALL PARTS OF THE WORK EXCEPT WHERE SPECIFICALLY DETAILED OR UNLESS NOTED OTHERWISE (U.N.O.)

2. THE STRUCTURAL DRAWINGS ILLUSTRATE THE NEW STRUCTURAL MEMBERS. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR NON-STRUCTURAL ITEMS WHICH REQUIRE SPECIAL PROVISIONS DURING THE CONSTRUCTION OF THE STRUCTURAL MEMBERS.

3. REFER TO EXISTING BUILDING DRAWINGS FOR EXISTING MEMBER SIZE AND LOCATION AND VIF.

4. REFER TO ARCHITECTURAL DRAWINGS FOR FLOOR DEPRESSIONS, CHANGES OF ELEVATION, EDGE OF SLAB, SIZE AND LOCATION OF OPENINGS, SLOPES, DRAINS, CURBS, PADS, EMBEDDED ITEMS, CHAMFERS, NON-BEARING PARTITIONS, DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, ETC. REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, OPENINGS, HANGERS, TRENCHES, CONCRETE INSERTS, AND ANCHORAGE FOR PIPES, DUCTS, EQUIPMENT, AND FIXTURES.

5. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHALL VERIFY ALL DIMENSIONS AND CONDITIONS WHICH IMPACT THE WORK. FIELD VERIFY SIZES, ELEVATIONS, HOLE LOCATIONS, ETC. PRIOR TO FABRICATION.

6. DRAWING DIMENSIONS ARE TO FACE OF FINISH, JOINT CENTERLINE OR COLUMN GRID CENTERLINE UNLESS NOTED OTHERWISE. DO NOT SCALE THE DRAWINGS.

7. CONTRACTOR SHALL CAREFULLY REVIEW THE DRAWINGS TO IDENTIFY THE SCOPE OF WORK REQUIRED. VISIT THE SITE TO RELATE THE SCOPE OF WORK TO EXISTING CONDITIONS AND DETERMINE THE EXTENT TO WHICH THOSE CONDITIONS AND PHYSICAL SURROUNDINGS WILL IMPACT THE WORK.

8. EXISTING CONDITIONS AS SHOWN ON THESE PLANS ARE FOR REFERENCE ONLY. CONTRACTOR IS REQUIRED TO FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO CONSTRUCTION. CONTRACTOR SHALL REPORT CONDITIONS THAT CONFLICT WITH THE CONTRACT DOCUMENTS TO THE OWNER 'S REPRESENTATIVE. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION FROM THE OWNER 'S REPRESENTATIVE.

9. THE CONTRACTOR SHALL RESOLVE ANY CONFLICTS ON THE DRAWINGS OR IN THE SPECIFICATIONS WITH THE OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.

10. ANY DEVIATION, MODIFICATION & SUBSTITUTION FROM THE APPROVED SET OF STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER FOR REVIEW/APPROVAL PRIOR TO ITS USE OR INCLUSION ON THE SHOP DRAWINGS & PRIOR TO PROCEEDING WITH THE WORK.

11. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY SHORES, BRACES AND GUYS REQUIRED TO SUPPORT ALL LOADS TO WHICH THE BUILDING STRUCTURE AND COMPONENTS, SOILS, OTHER STRUCTURES AND UTILITIES MAY BE SUBJECTED DURING CONSTRUCTION. SHORING SYSTEMS SHALL BE DESIGNED AND STAMPED BY A CIVIL ENGINEER LICENSED IN THE STATE OF CALIFORNIA.

12. THE CONTRACTOR SHALL PROVIDE MEANS, METHOD, TECHNIQUES, SEQUENCE AND PROCEDURE OF CONSTRUCTION AS REQUIRED.

13. SITE VISITS PERFORMED BY THE OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER DO NOT INCLUDE INSPECTIONS OF MEANS AND METHODS OF CONSTRUCTION PERFORMED BY CONTRACTOR.

14. THE CONTRACTOR SHALL PROTECT ALL WORK, MATERIALS AND EQUIPMENT FROM DAMAGE AND SHALL PROVIDE PROPER STORAGE FACILITIES FOR MATERIALS AND EQUIPMENT DURING CONSTRUCTION.

15. CONTRACTORS SHALL REVIEW SHOP DRAWINGS FOR COMPLETENESS AND COMPLIANCE WITH CONTRACT DOCUMENTS. CONTRACTOR SHALL STAMP SHOP DRAWINGS PRIOR TO SUBMISSION TO OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER .

16. REVIEW OF THE SHOP DRAWINGS SHALL NOT BE CONSTRUED AS AN AUTHORIZATION TO DEVIATE FROM CONTRACT DOCUMENTS.

17. SHOP DRAWINGS WILL NOT BE PROCESSED DUE TO INCOMPLETENESS, LACK OF COORDINATION WITH RELEVANT PORTION OF CONTRACT DOCUMENTS, LACK OF CALCULATIONS IF REQUIRED AND WHERE DEVIATIONS, MODIFICATIONS AND SUBSTITUTIONS ARE INDICATED WITHOUT PRIOR WRITTEN APPROVAL FROM OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER .

18. ALLOW TEN WORKING DAYS FOR PROCESSING SHOP DRAWINGS AFTER RECEIPT. ALLOW FIVE WORKING DAYS FOR PROCESSING RFI'S AFTER RECEIPT.

MECHANICAL AND ADHESIVE ANCHORS:

1. ALL MECHANICAL AND ADHESIVE ANCHORS TO HAVE CURRENT ICC-ES OR IAPMO-ES REPORTS FOR USE ON THIS PROJECT. A COPY OF THE LOS ANGELES RESEARCH REPORT AND OR CONDITIONS OF LISTING SHALL BE MADE AVAILABLE AT THE JOB SITE. CONTRACTOR SHALL OBTAIN SEOR'S APPROVAL TO SUBSTITUTE PRODUCTS OF OTHER MANUFACTURER'S, SUBJECT TO DEMONSTRATING EQUIVALENT TENSION AND SHEAR STRENGTH TO THE SPECIFIED ANCHOR, UNDER PROJECT INSTALLATION CONDITIONS.

MECHANICAL ANCHORS INTO CONCRETE				
	HILTI	HILTI	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE
ANCHOR	KB-TZ2	KH-EZ	STRONG-BOLT 2	TITEN HD
ICC-ESR OR IAPMO-ES	4266	3027	3037	2713

MECHANICAL ANCHORS INTO GROUT-FILLED MASONRY				
	HILTI	HILTI	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE
ANCHOR	KB-TZ2	KH-EZ	STRONG-BOLT 2	TITEN HD
ICC-ESR OR IAPMO-ES	4561	3056	240	1056

ADHESIVE ANCHORS AND DOWELS INSTALLED INTO CONCRETE				
	HILTI	HILTI	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE
ANCHOR	HIT-RE 500 V3	HIT-HY 200 V3	SET-3G	SET-XP
ICC-ESR OR IAPMO-ES	3814	3027	4057	2508

ADHESIVE ANCHORS AND DOWELS INSTALLED INTO GROUT-FILLED MASONRY				
	HILTI	HILTI	SIMPSON STRONG-TIE	SIMPSON STRONG-TIE
ANCHOR	HIT-HY 270	HIT-HY 200 V3	SET-3G	SET-XP
ICC-ESR OR IAPMO-ES	4143	4878	4844	265

7. ADHESIVE ANCHORS: ***ASTM F1554 GR 36 THREADED RODS***, U.N.O.

8. ADHESIVE DOWELS: ***ASTM A615 GRADE 60 REINFORCING STEEL***, U.N.O.

9. ALL ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH ***ICC, IAMPO,*** AND MANUFACTURER RECOMMENDATIONS.

10. PRIOR TO ALL DRILLING OR CORING, THE CONTRACTOR SHALL (1) VERIFY THE EXISTING CONCRETE OR MASONRY THICKNESS TO PREVENT DAMAGE TO THE OPPOSITE FACE OF CONCRETE AND MAINTAIN 1-1/2" CLEAR COVER U.N.O., AND (2) IDENTIFY EXISTING REINFORCING LOCATIONS BY PACHOMETER, PROBING, CHIPPING, ETC TO AVOID DAMAGE EXISTING REINFORCING.

11. CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL OR ADHESIVE ANCHORS. AT CONTRACTOR OPTION, OVERSIZED HOLES AND WELDED PLATE WASHERS CAN BE USED IN LIEU OF STANDARD DIAMETER HOLES.

12. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2 ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE DOWEL AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ***OWNER'S REPRESENTATIVE/ARCHITECT/ENGINEER*** WILL DETERMINE A NEW LOCATION.

13. ANCHORS SHALL BE PROOF-TESTED BY OWNER'S TESTING AND INSPECTION AGENCY. TEST ***20%*** OF ALL ANCHORS. ONCE ***20*** CONSECUTIVE ANCHORS OF THE SAME TYPE INTO STRUCTURAL ELEMENTS OF THE SAME TYPE PASS THE TEST REQUIREMENTS, ANCHOR TESTING MAY BE REDUCED TO ***10%*** OF ALL ANCHORS.

14. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:

A. HYDRAULIC RAM METHOD: THE ANCHOR MUST NOT EXHIBIT OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE AND SLEEVE TYPE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE. DROP-IN ANCHORS ARE ONLY TO BE TESTED WITH THIS METHOD.
B. TORQUE WRENCH METHOD: THE APPLICABLE TEST TORQUE FOR WEDGE OR SLEEVE-TYPE ANCHORS, MUST BE REACHED WITHIN THE FOLLOWING LIMITS, ONE-HALF (1/2) TURN OF THE NUT; ONE QUARTER (1/4) TURN OF THE NUT FOR THE 3/8" SLEEVE ANCHOR ONLY.
C. IF ANY ANCHOR FAILS TESTING, ALL ANCHORS OF THE SAME CATEGORY, NOT PREVIOUSLY TESTED, SHALL BE TESTED UNTIL ***20*** CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENTS. THE INITIAL TESTING FREQUENCY SHALL THEN BE RESUMED.
D. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY THE FIXTURE(S).
E. TESTING SHOULD OCCUR 24 HOURS MIN. AFTER INSTALLATION OF ANCHORS.

STRUCTURAL SHEET LIST - SEISMIC

S001	GENERAL NOTES
S002	GENERAL NOTES
S101	UPPER FRAMING PLAN
S102	ROOF FRAMING PLAN
S201	DETAIL

BASIS OF DESIGN:

1. PROJECT DESCRIPTION:
THIS PROJECT CONSISTS OF VOLUNTARY SEISMIC STRENGTHENING OFAN EXISTING 3-STORY MEDICAL OFFICE BUILDING WITH CMU SHEAR WALLS. STRENGTHENING IS LIMITED TO NEW STEEL COLLECTORS AND STRENGTHENING DIAPHRAGM-TO-WALL CONNECTIONS AT THE UPPER FLOOR AND ROOF.

2. FOR EXISTING CONSTRUCTION, REFER TO EXISTING STRUCTURAL DRAWINGS BY THEODORE E. ANVICK DATED DECEMBER 1975, AND EXISTING SPANCRETE DRAWINGS BY SPANCRETE OF CALIFORNIA DATED APRIL 1976.

3. ALL NEW CONSTRUCTION SHALL COMPLY WITH THE CONTRACT DOCUMENTS AND THE 2025 EDITION OF THE CALIFORNIA BUILDING CODE, WITH 2025 CITY OF REDONDO BEACH AMENDMENTS.

4. SEISMIC DESIGN INFORMATION:
SEISMIC HAZARD FACTORS:
SITE CLASS D

BSE-1E
SXS = 0.92
SX1 = 0.58

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STATE OF CALIFORNIA

Ryan Wilkerson

BCHD Beach Cities Health District
A Public Agency

SEISMIC STRENGTHENING

510 N PROSPECT AVE
REDONDO BEACH, CA 90277

ISSUE DESCRIPTION

	DESIGN DEVELOPMENT	11/14/2025
	PLAN CHECK	01/15/2026

SHEET TITLE
GENERAL NOTES

SHEET NUMBER
S001

FRAMING SYMBOLS AND NOTATIONS:

C-X — INDICATES CONCRETE COLUMN TYPE: SEE COLUMN SCHEDULE, ***ON X/SX.XX***

F-X — INDICATES COLUMN FOOTING TYPE: SEE COLUMN FOOTING SCHEDULE, ***ON X/SX.XX***

-XX'-XX" — INDICATES TOP OF FOOTING (TOF): WHERE NOT NOTED, TOP OF FOOTING SHALL BE BELOW LOWEST ADJACENT TOP OF FINISH OR GRADE ***-1'-6"*** TYP.

S-----S	INDICATES STEP IN FOOTING DETAIL, SEE ***SX.XX***
GB-X	INDICATES CONCRETE GRADE BEAM TYPE, SEE ***SX.XX***
B-X	INDICATES CONCRETE BEAM, SEE ***SX.XX***
c=	INDICATES REQUIRED MID-SPAN CAMBER IN INCHES
cc=	INDICATES REQUIRED CAMBER AT END OF CANTILEVERED BEAM
	INDICATES TWO WAY CONCRETE SLAB, SEE ***SX.XX***
	INDICATES 1-WAY CONCRETE SLAB TYPE, SEE ***SX.XX***
T/SLAB X'-X"	INDICATES ELEVATION OF TOP OF STRUCTURAL CONCRETE.
	INDICATES SLAB STEP
	INDICATES CHANGE IN SLOPE
	INDICATES BUILT-UP SLAB, SEE ***X/SX.XX***
Wx [XX] C=Y"	(N) COMPOSITE STEEL BEAM, SEE ***X/SXXX*** Wx = BEAM SECTION [XX] = NUMBER OF STUDS. PROVIDE AT ***12"*** U.N.O. Y" = MIDSPAN CAMBER
X	INDICATES (N) STEEL BEAM PENETRATION PER ***X/SX.XX***
	INDICATES (N) KICKER BRACE PER ***X/SX.XX*** INDICATES LOW END OF KICKER
▶	INDICATES STEEL MOMENT FRAME CONNECTION, SEE ***X/SX.XX***
▷	INDICATES CANTILEVER MOMENT CONNECTION, SEE ***X/SX.XX***
●	INDICATES LFRS DRAG CONNECTION, SEE ***X/SX.XX***
(-X") =	INDICATES ELEVATION OF TOP OF STEEL (T/STL) BELOW TOP OF METAL DECKING (T/DECK). IF NO ELEVATION IS INDICATED, LOCATE T/STL BELOW BOTTOM OF METAL DECKING (B/DECK).
	INDICATES VERCO METAL DECK. SEE TYPICAL METAL DECK DETAILS AND GENERAL NOTES FOR ADDITIONAL INFO.
SB 11'-6"	INDICATES ***2 SIDED SURE-BOARD SHEAR WALL SERIES 200W PANEL OVER 600S162 146A STUDS @ 16" O.C. W/#10 SMS @ 6" O.C. @ ALL EDGES, BOUNDARIES & FIELD.***

C/A = INDICATES COLUMN ABOVE

C/B = INDICATES COLUMN BELOW

LP = INDICATES LOW POINT = XX'-X"

HP = INDICATES HIGH POINT = XX'-X"

T/SLAB = INDICATES TOP OF STRUCTURAL CONCRETE SLAB

B/SLAB = INDICATES BOTTOM OF STRUCTURAL CONCRETE SLAB

T/DECK = INDICATES TOP OF METAL DECK

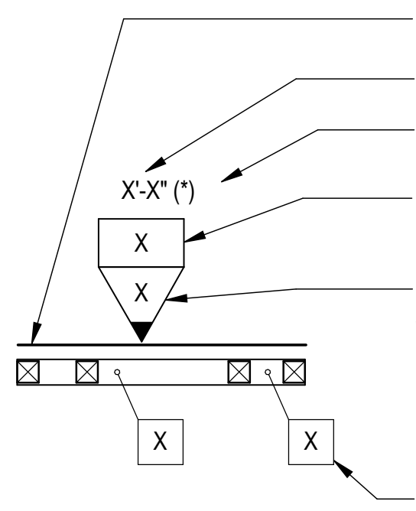
B/DECK = INDICATES BOTTOM OF METAL DECK

T/STL = INDICATES TOP OF STRUCTURAL STEEL.

X X	INDICATES DETAIL CALLOUT.
X X	INDICATES ELEVATION CALLOUT.
U.N.O.	INDICATES UNLESS NOTED OTHERWISE.
S.O.G.	INDICATES SLAB ON GRADE
Ld	INDICATES CLASS "A" SPLICE/DEVELOPMENT LENGTH PER ***X/SX.XX*** FOR CONCRETE OR ***X/SX.XX*** FOR CMU.
CLASS "B"	INDICATES CLASS "B" SPLICE/DEVELOPMENT LENGTH PER ***X/SX.XX***
Ldh	DENOTES HOOK DEVELOPMENT LENGTH PER ***X/SX.XX***
C.J.	INDICATES CONCRETE COLD JOINT, ROUGHEN TO AN AMPLITUDE OF |2" U.N.O.
E.W.	DENOTES EACH WAY
E.F.	DENOTES EACH FACE
TYP.	DENOTES TYPICAL
MAX.	DENOTES MAXIMUM
MIN.	DENOTES MINIMUM
VERT. OR V	DENOTES VERTICAL
HORIZ. OR H	DENOTES HORIZONTAL
EQ.	DENOTES EQUAL
CONT.	DENOTES CONTINUOUS
S.A.D.	DENOTES "SEE ARCHITECTURAL DRAWINGS".
NLB	DENOTES NON-LOAD BEARING
LB	DENOTES LOAD BEARING
DBA	DENOTES DEFORMED BAR ANCHOR NELSON "D2L" OR EQUIVALENT.

FRAMING SYMBOLS AND NOTATIONS (CONT.):

(E) Wx	(E) STEEL BEAM
(E) Wx MF	(E) STEEL MOMENT FRAME BEAM
	(N) CONC/STL COLUMN ABOVE
	(N) CONC/STL COLUMN BELOW
	(E) CONC/STL COLUMN ABOVE
	(E) CONC/STL COLUMN BELOW
	(E) CIP CONCRETE WALL ABOVE
	(E) CIP CONCRETE WALL BELOW
	(N) OPENING IN (E) CONC. WALL
	(N) CIP/SHOTCRETE CONCRETE WALL
	INDICATES LOAD BEARING (LB) CMU WALL. SEE SHEET ***SXXX***
	INDICATES NON-LOAD BEARING (NLB) CMU WALL. SEE SHEET ***SXXX***
	(E) CMU WALL ABOVE
	SOLID LINE DENOTES ADD'L TOP REINFORCEMENT
	DASH LINE DENOTES ADD'L BOTTOM REINFORCEMENT



DENOTES SHEAR WALL EXTENTS

MIN. SHEAR WALL LENGTH

INDICATES PERFORATED WALL PER ***X/SX.XX***

INDICATES HOLDOWN TYPE PER ***X/SX.XX***

DENOTES SHEAR WALL TYPE SEE SHEAR WALL SCHEDULE ON ***X/SX.XX***

INDICATES HOLDOWN TYPE PER X/SX.XX WHERE WALL LENGTH VARIES OR IS DISCONTINUOUS.

PX	DENOTES WOOD POST ABOVE PER SCHEDULE
PB	DENOTES WOOD POST BELOW PER SCHEDULE

ABBREVIATIONS:

&	AND
@	AT
CL	CENTER LINE
PL	PLATE
A.B.	ANCHOR BOLT
ABV	ABOVE
ADD'L	ADDITIONAL
ALT	ALTERNATE
A.R.	ANCHOR ROD
ARCH	ARCHITECTURAL
BF	BRACE FRAME
BLW	BELOW
BM	BEAM
A.N.	BOUNDARY NAILING
A.O.	BOTTOM OF
BOTT.	BOTTOM
BRBF	BUCKING-RESTRAINED BRACE FRAME
BTWN	BETWEEN
C.J.	CONSTRUCTION JOINT
CJP	COMPLETE JOINT PENETRATION
CLR	CLEAR
C.M.U	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
COND	CONDITION
CONN	CONNECTION
CONT	CONTINUOUS
C.S.	CONCRETE SCREW
d	PENNY (NAIL SIZE AS IN 10d)
D.B.A.	DEFORMED BAR ANCHOR
DBL	DOUBLE
D.C.W.	DEMAND-CRITICAL WELD PER AISC
DET.	DETAIL
D.F.	DOUGLAS FIR
DIA OR Ø	DIAMETER
DWG	DRAWING(S)
DWL	DOWEL
(E)	EXISTING
EA.	EACH
EBF	ECCENTRIC BRACED FRAME
E.F.	EACH FACE
E.J.	EXPANSION JOINT
EL. OR ELEV.	ELEVATION
E.N.	END NAILING
E.P.S.	EXPANDED POLYSTYRENE
E.S.	EACH SIDE
EQ.	EQUAL
E.W.	EACH WAY
EXA	EXPANSION ANCHOR
FIN.	FINISH
FLG.	FLANGE
FLR	FLOOR
F.N.	FIELD NAILING
FNDN	FOUNDATION
F.O.C.	FACE OF CONCRETE
F.O.W.	FACE OF WALL
FRMG	FRAMING
F.S.	FAR SIDE
FTG	FOOTING
GA	GAUGE
GALV.	GALVANIZED
G.B.	GRADE BEAM
G.L.	GLULAM
GR	GRADE
H OR HORIZ	HORIZONTAL
HDR	HEADER
H.S.	HEADED STUD
HSB	HIGH STRENGTH BOLT
HSS	HOLLOW STRUCTURAL SECTION
ld	DEVELOPMENT LENGTH
INFO	INFORMANTION
I.F.	INSIDE FACE
JST	JOIST
JT	JOINT
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG/SIDE VERTICAL
L.S.	LAG SCREW
MAX	MAXIMUM
M.B.	MACHINE BOLT
MECH	MECHANICAL
MIN	MINIMUM
MF	MOMENT FRAME
(N)	NEW
N/A	NOT APPLICABLE
N.I.C.	NOT IN CONTRACT
NR	NOT REDUCIBLE
N.S.	NEAR SIDE
NTS	NOT TO SCALE
O.C.	ON CENTER
O.F.	OUTSIDE FACE
O.H.	OPPOSITE HAND
OPG OR OPNG	OPPENING
O.S.B.	ORIENTED STRAND BOARD
O.W.S.J.	OPEN WEB STEEL JOIST
PG	PLATE GIRDER
P.J.P.	PARTIAL JOINT PENETRATION
PL.	PLATE
PLWD OR PLY.	PLYWOOD
P.T.	POST TENSION OR PRESSURE TREATED
P.Z.	PROTECTED ZONE
R.D.	RESIN DOWEL
REINF.	REINFORCING
REQ.	REQUIRED
R.O.	ROUGH OPENING
RWD	REDWOOD
S.A.D.	SEE ARCHITECTURAL DRAWINGS
S.B.	SOLID BLOCKING
S.C.D.	SEE CIVIL DRAWINGS
SCHED.	SCHEDULE
SECT.	SECTION
S.E.D.	SEE ELECTRICAL DRAWINGS
SHT	SHEET(S)
SIM	SIMILAR
S.J.	SEISMIC JOINT
SLRS	SEISMIC LOAD RESISTING SYSTEM
S.M.D.	SEE MECHANICAL DRAWINGS
S.M.S.	SHEET METAL SCREW
S.P.D.	SEE PLUMBING DRAWINGS
SPECS	SPECIFICATIONS
SQ.	SQUARE
S.R.	SEISMIC RESISTING
STAGG.	STAGGERED
STD	STANDARD
STIFF.	SIFFENER
STL	STEEL
SYMM.	SYMMETRICAL
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
T.B.	TIE BEAM
T.F.F.	TOP OF FINISHED FLOOR
T.O.C.	TOP OF CONCRETE
T.O.F.	TOP OF FOOTING
T.O.S.	TOP OF STEEL
TG	TRANSFER GIRDER
THK	THICK
TYP	TYPICAL
U.A.	UNDERCUT ANCHOR
U.N.O.	UNLESS NOTED OTHERWISE
U/S	UNDERSIDE
V.I.F.	VERIFY IN FIELD
(V) OR VERT.	VERTICAL
W/	WITH
W/O	WITHOUT
W.P.	WORK POINT
W.S.	WOOD SCREW
W.T.S.	WELDED THREADED STUD
W.W.R	WELDED WIRE REINFORCEMENT

KEY NOTES:

1	***INDICATES KEYNOTE 1***
2	***INDICATES KEYNOTE 2***

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Exp. 06/30/26
Ryan Wilkerson

BCHD Beach Cities Health District
A Public Agency

SEISMIC STRENGTHENING

510 N PROSPECT AVE
REDONDO BEACH, CA 90277

ISSUE DESCRIPTION

	DESIGN DEVELOPMENT	11/14/2025
	PLAN CHECK	01/15/2026

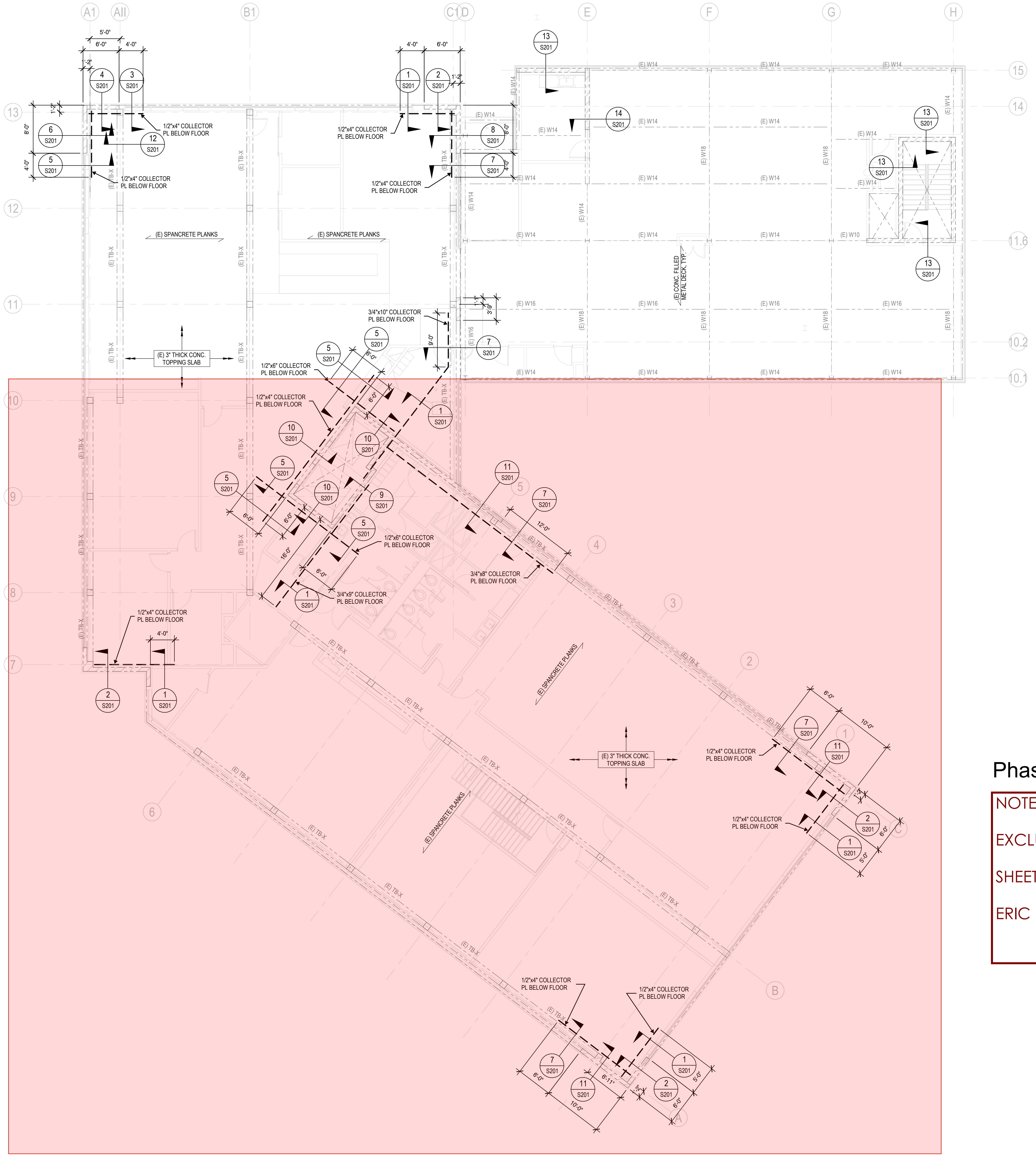
SHEET TITLE

GENERAL NOTES

SHEET NUMBER

S002

1/14/2026 3:03:14 PM



Phase 1 - Early Package

NOTE:

EXCLUDED ON PROPOSAL: PHASE 1 SEISMIC UPGRADE:

SHEET S101 - ONLY BID WHAT IS OUTSIDE THE RED BOX

ERIC ELVIRA, 03/27/26

1 S101 UPPER FRAMING PLAN

SCALE: 1/8" = 1'-0"

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510 N PROSPECT AVE
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ISSUE DESCRIPTION

DESIGN DEVELOPMENT	11/14/2025
PLAN CHECK	01/15/2026

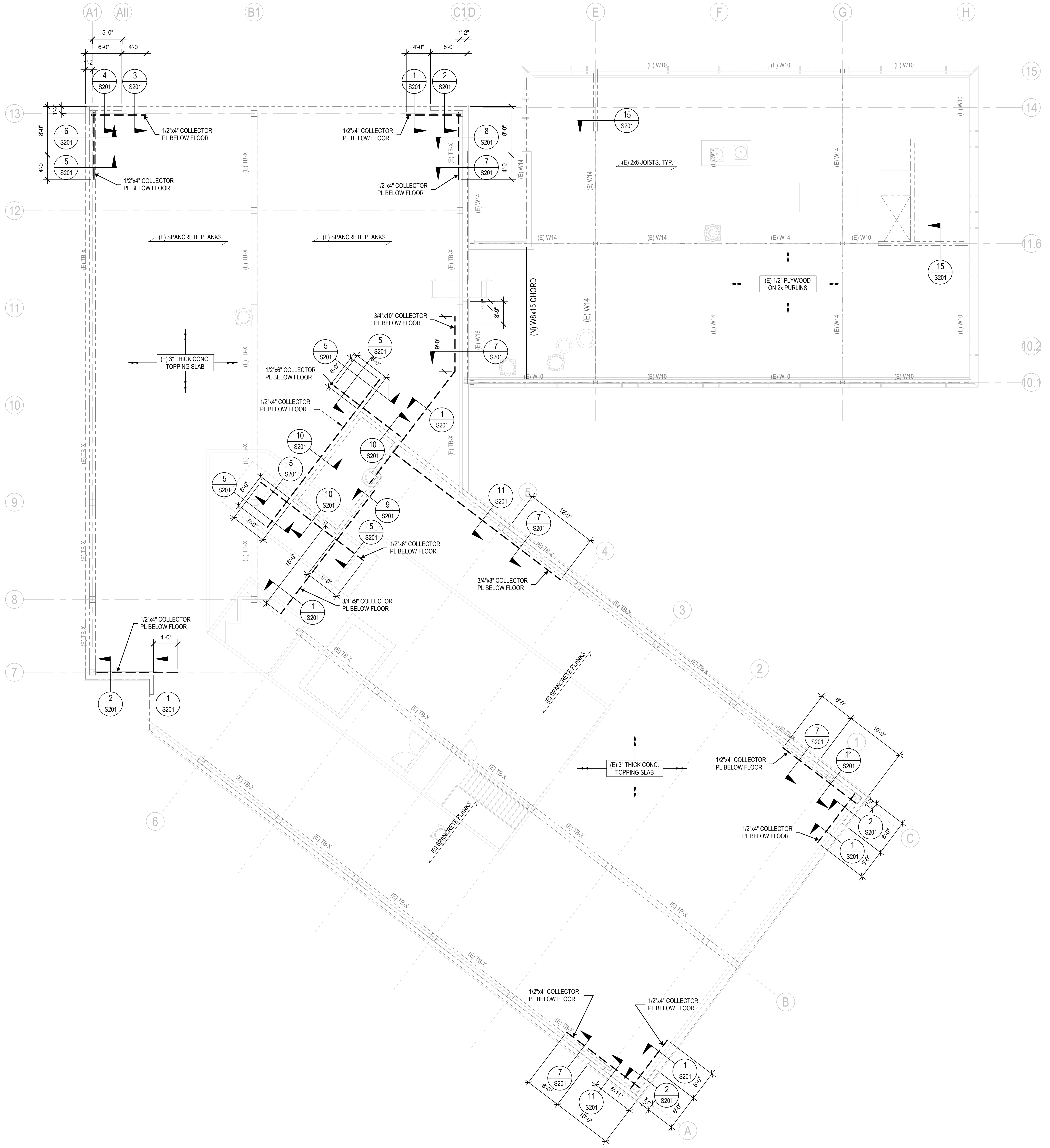
SHEET TITLE

UPPER FRAMING PLAN

SHEET NUMBER

S101

1/14/2026 3:03:15 PM



1 S102 **ROOF FRAMING PLAN**

SCALE: 1/8" = 1'-0"

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ISSUE DESCRIPTION

	DESIGN DEVELOPMENT	11/14/2025
	PLAN CHECK	01/15/2026

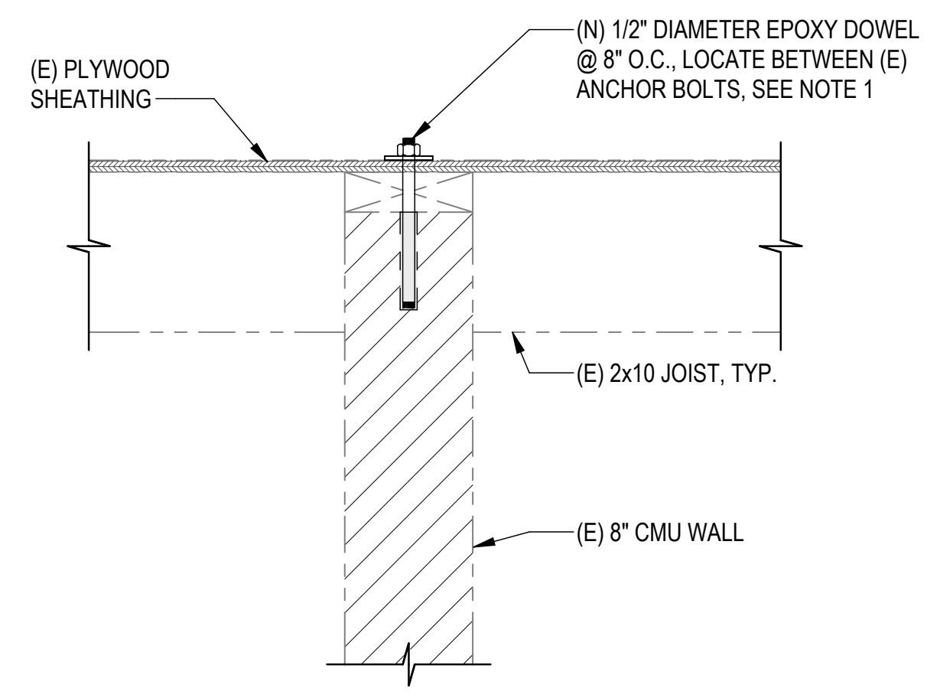
SHEET TITLE

ROOF FRAMING PLAN

SHEET NUMBER

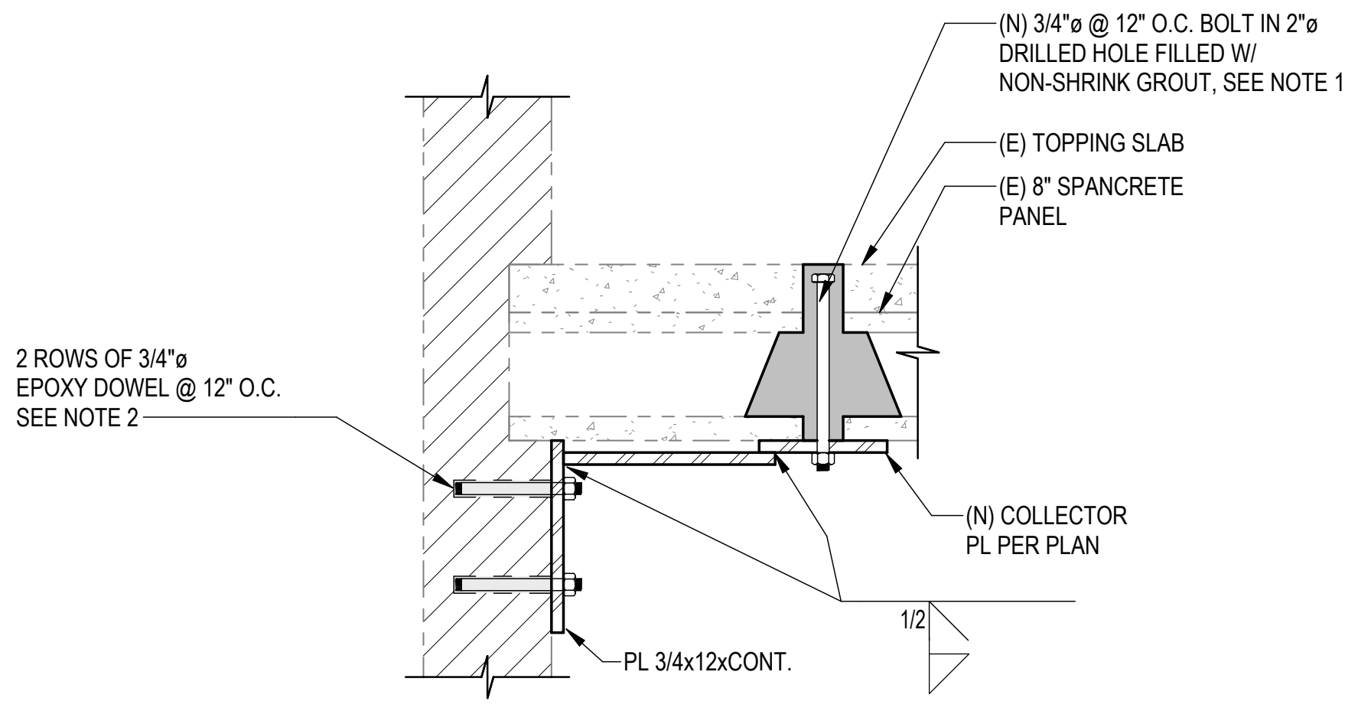
S102

1/14/2026 3:03:16 PM



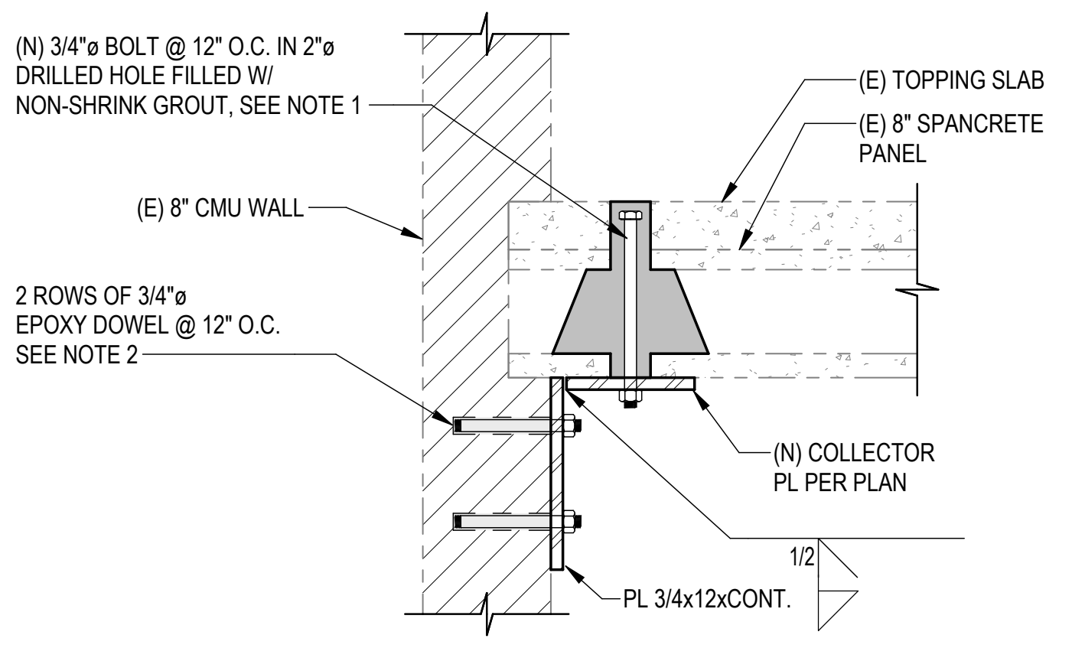
NOTES:
1. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

15 DIAPHRAGM-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



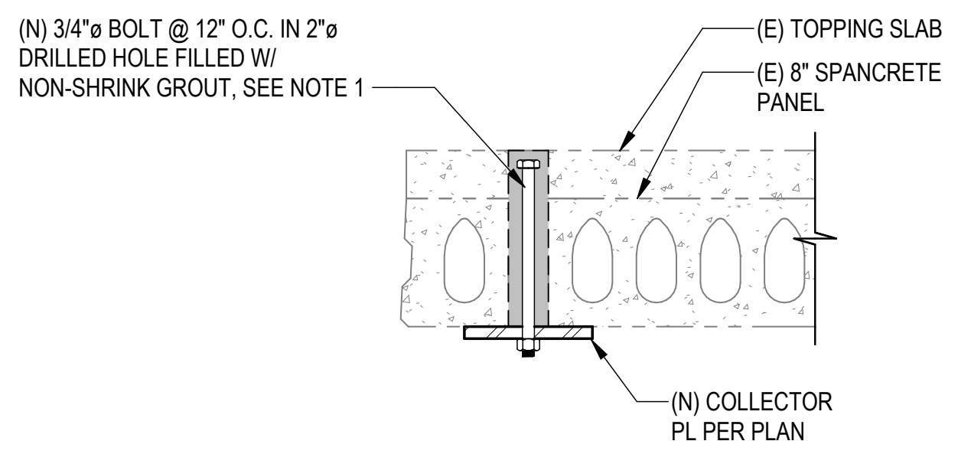
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.
2. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL AND BEAMS.

11 COLLECTOR-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



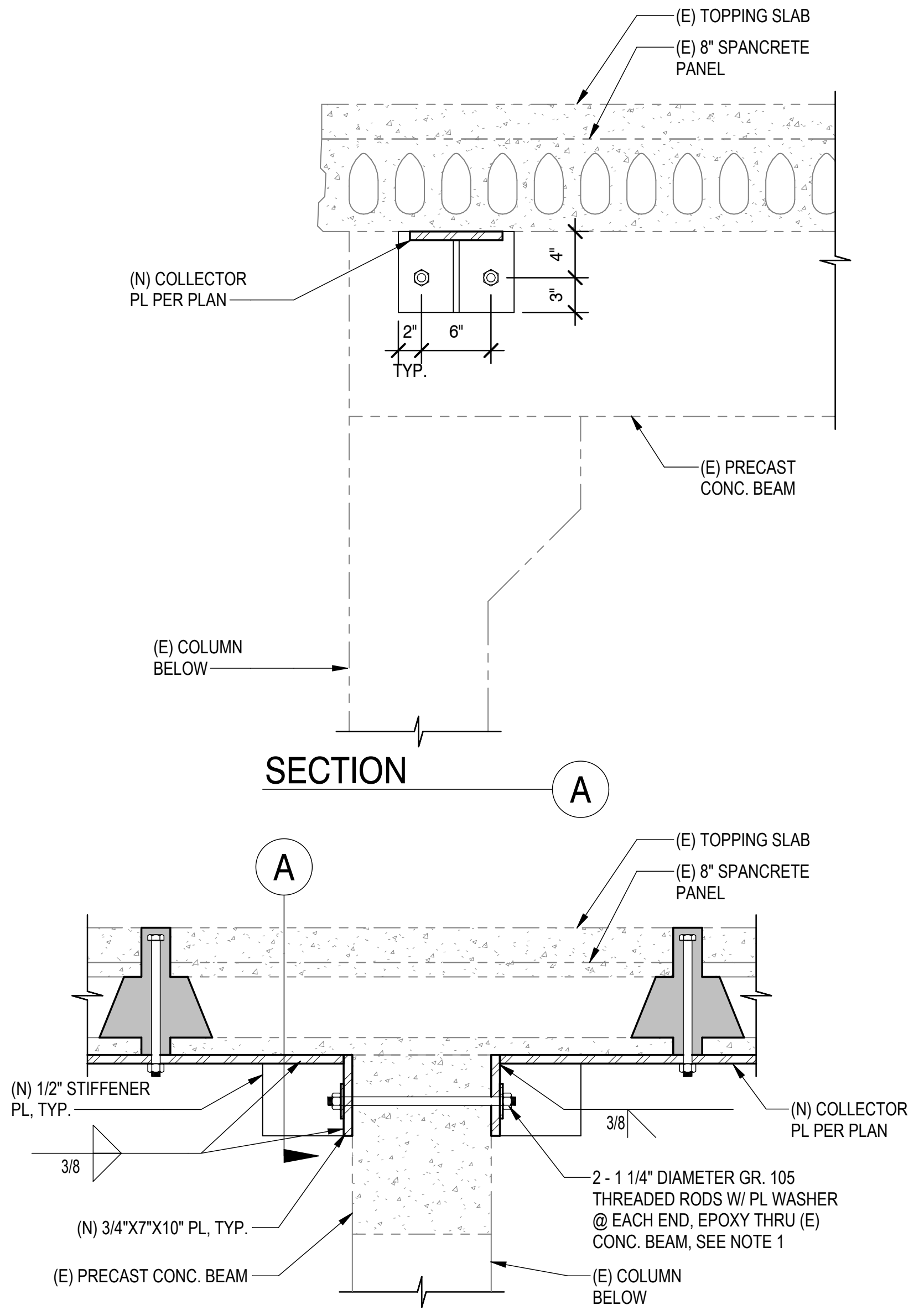
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.
2. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

6 COLLECTOR-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



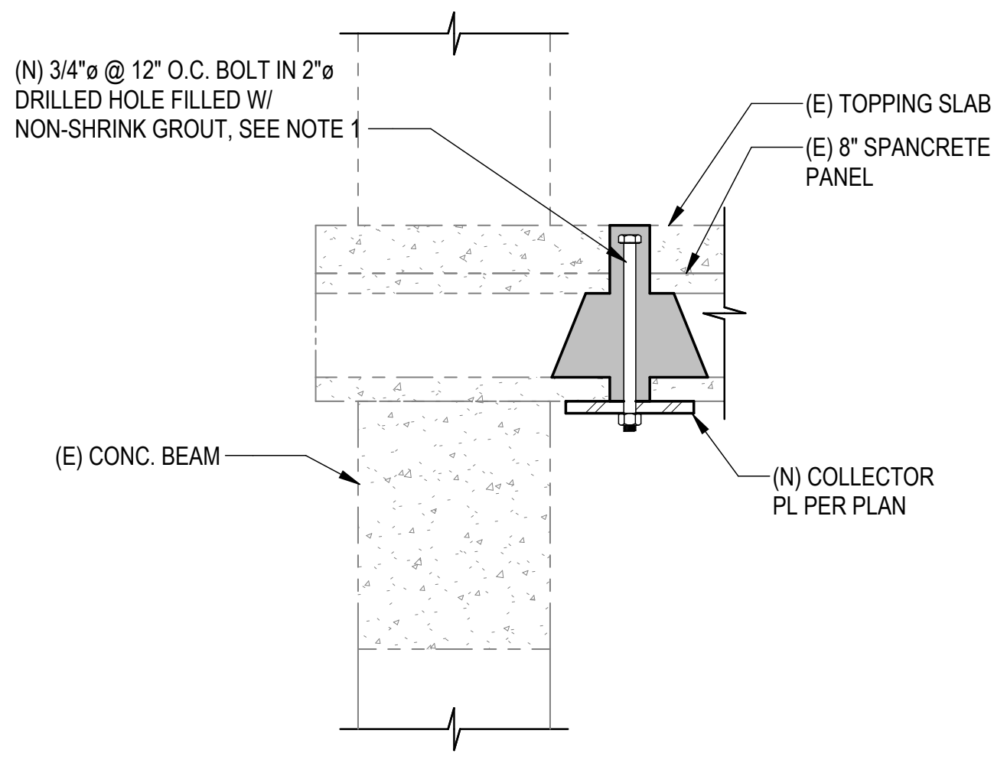
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.

1 COLLECTOR SECTION
S201 SCALE: 1" = 1'-0"



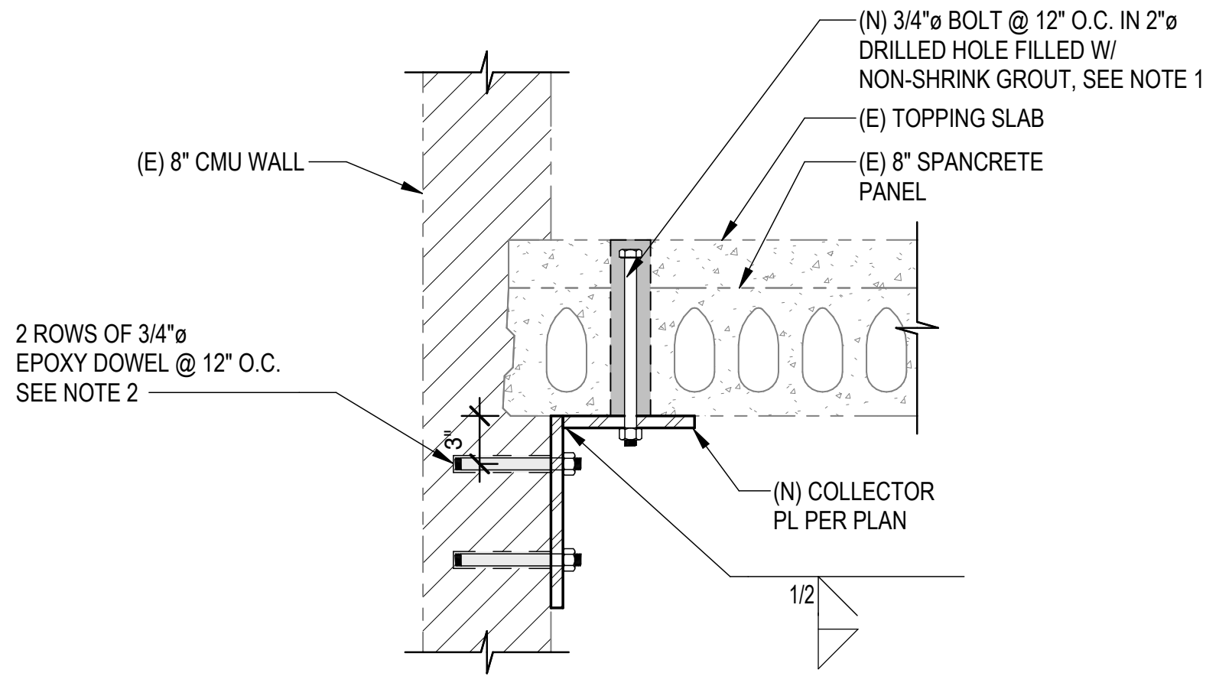
NOTES:
1. DRILLED HOLES SHALL BE LOCATED TO MISS BEAM REINFORCEMENT.

12 COLLECTOR THRU CONC. BEAM
S201 SCALE: 1" = 1'-0"



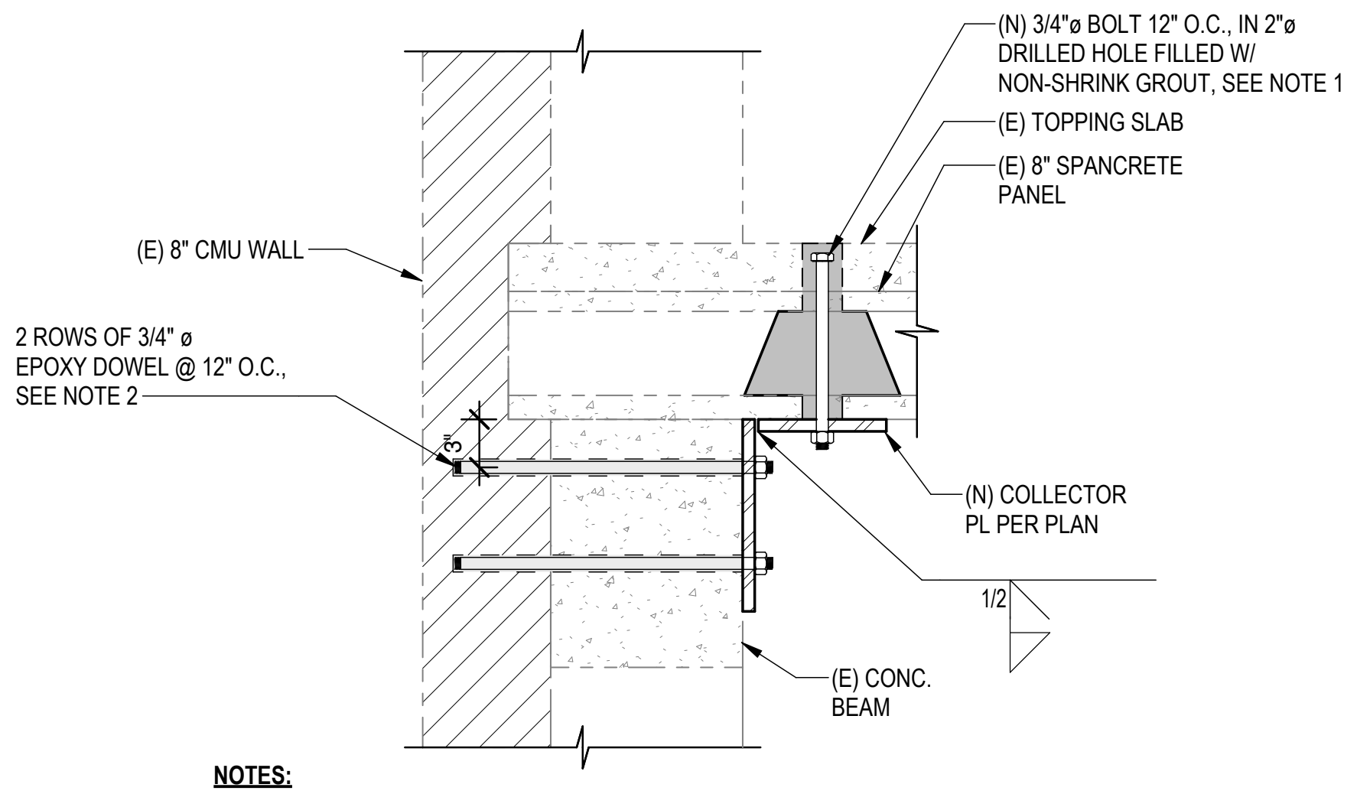
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.

7 COLLECTOR SECTION
S201 SCALE: 1" = 1'-0"



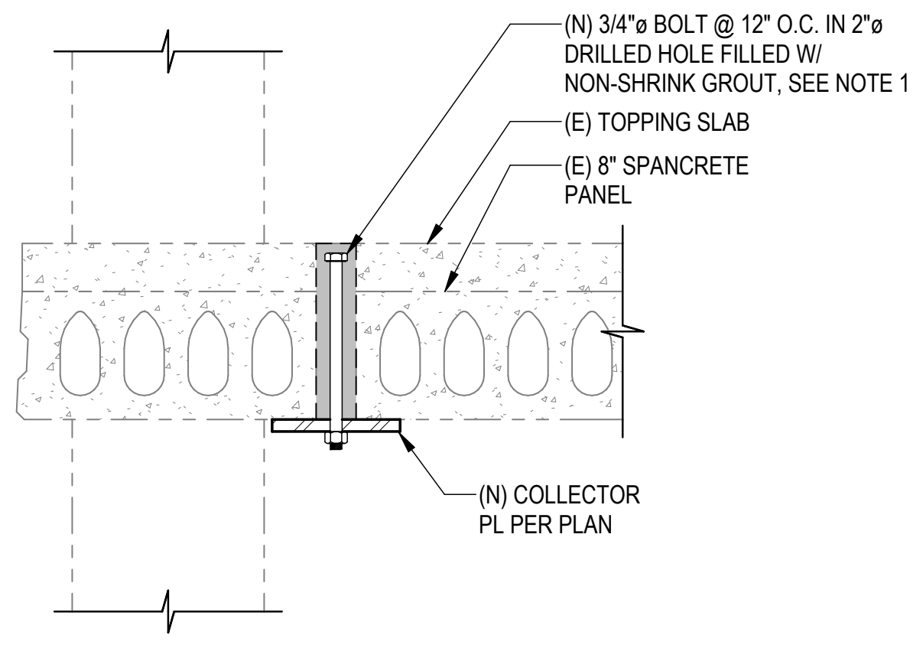
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.
2. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

2 COLLECTOR-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



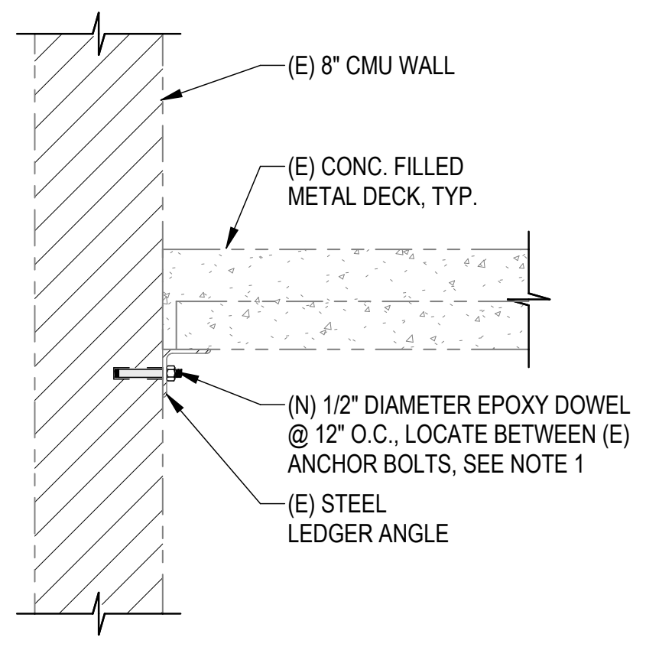
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.
2. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL AND BEAM.

8 COLLECTOR-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



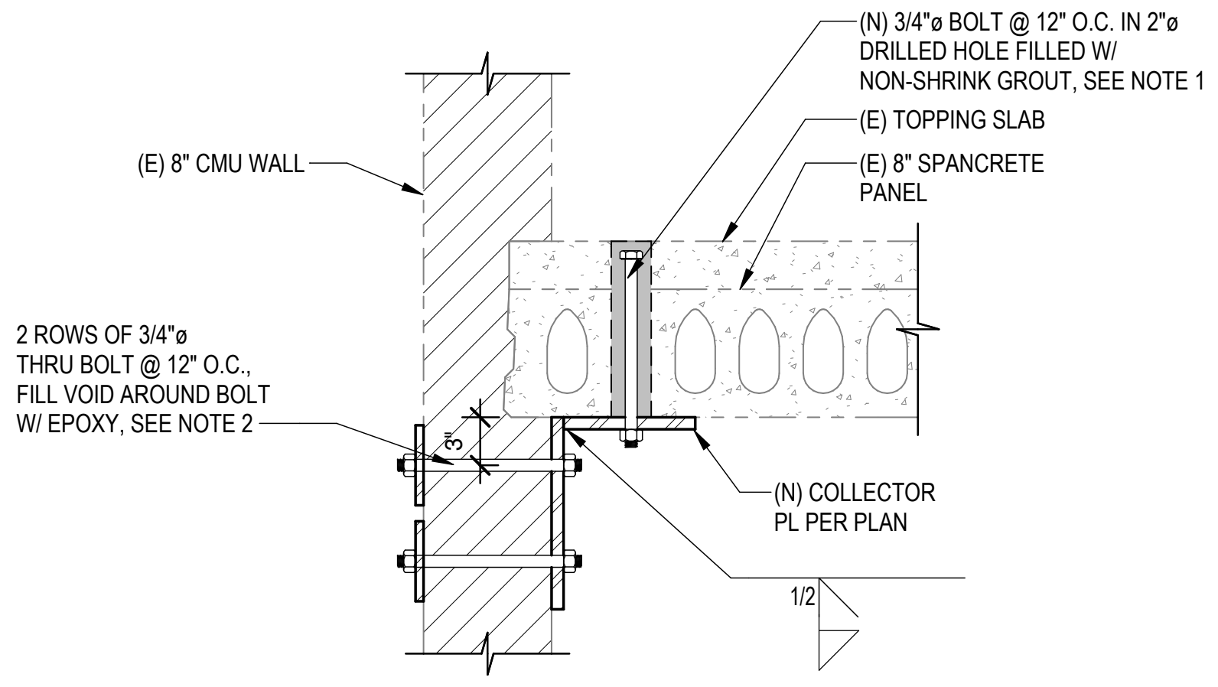
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.

3 COLLECTOR SECTION
S201 SCALE: 1" = 1'-0"



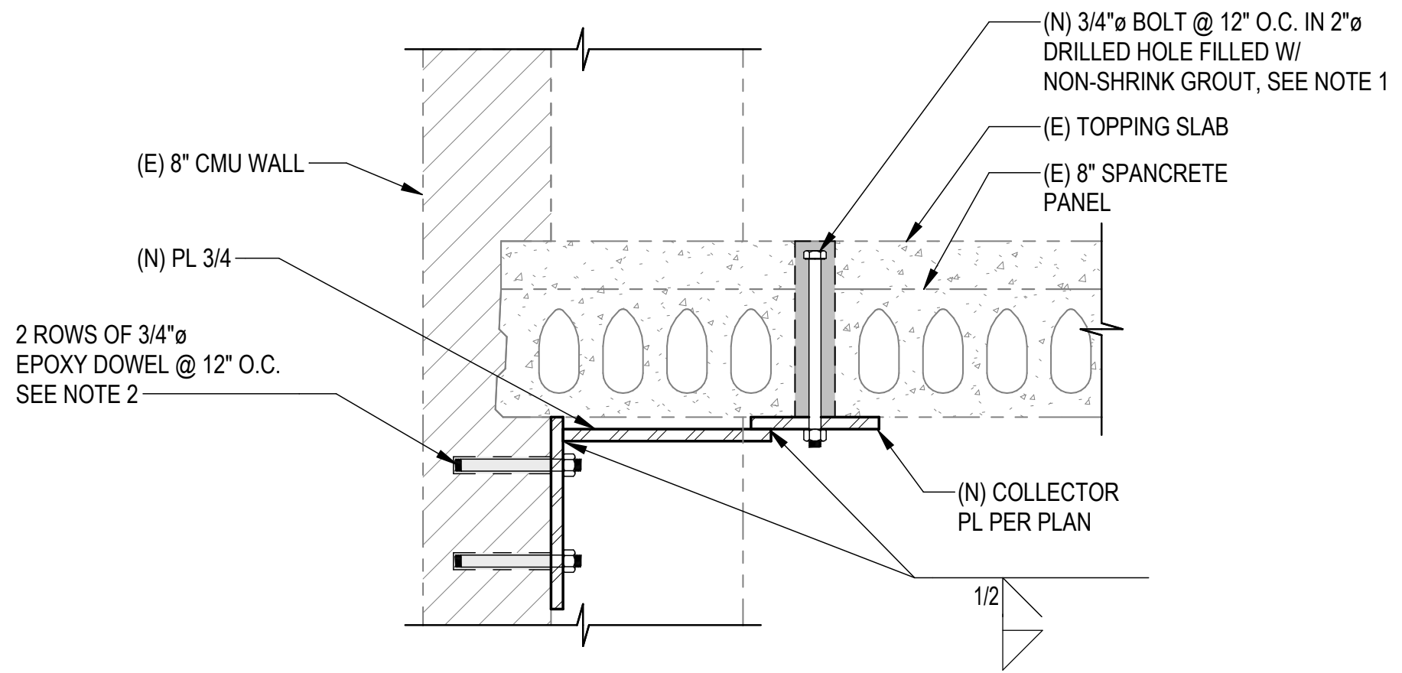
NOTES:
1. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

13 DIAPHRAGM-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



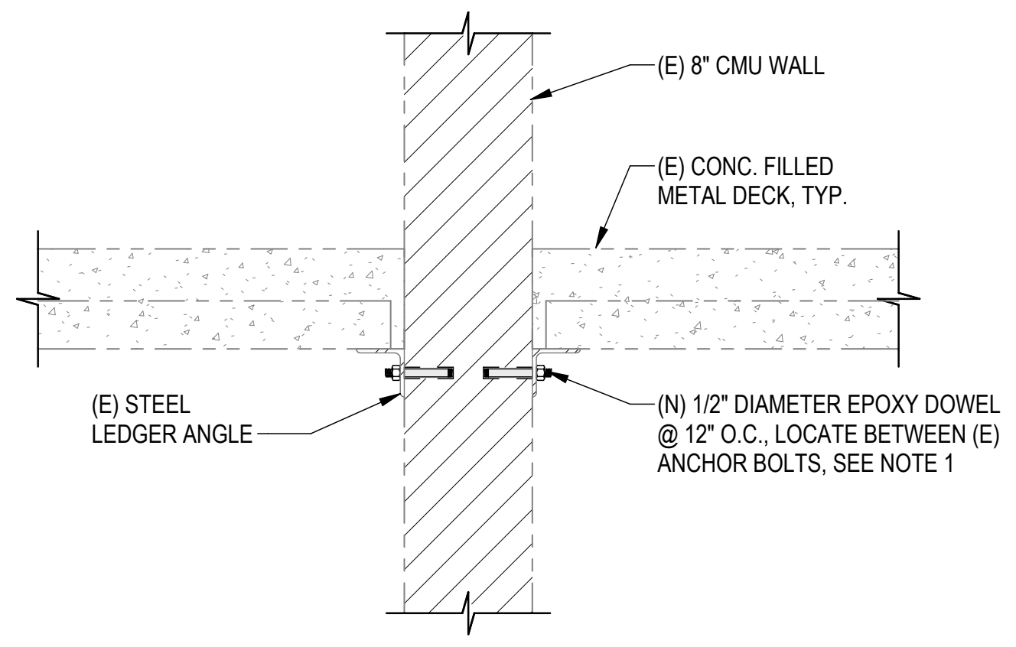
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.
2. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

9 COLLECTOR-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



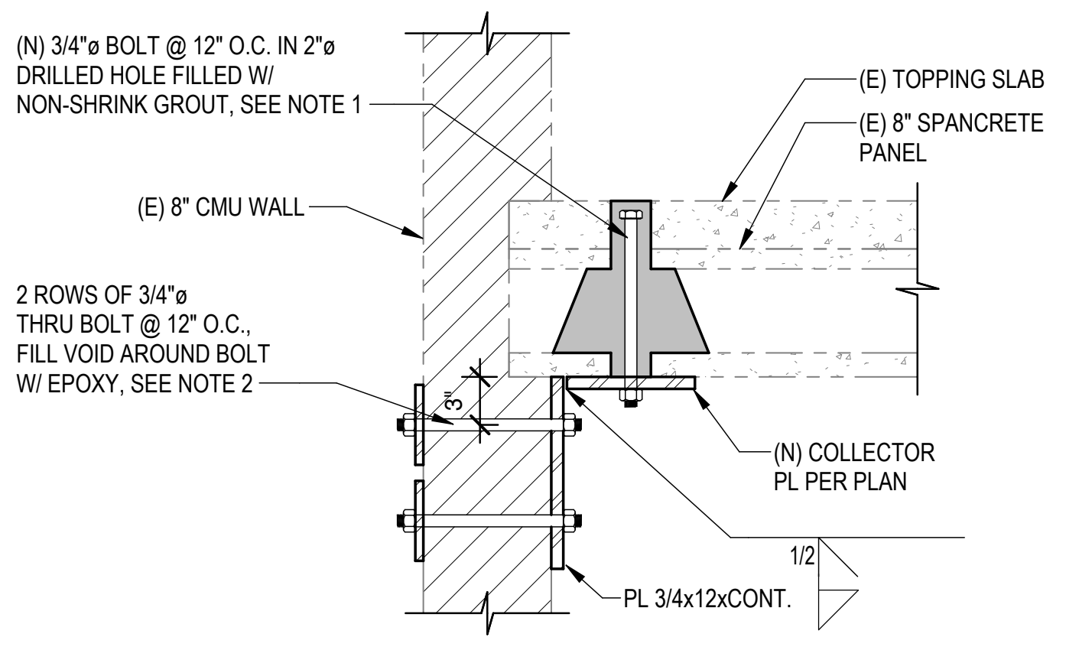
NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.
2. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

4 COLLECTOR-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



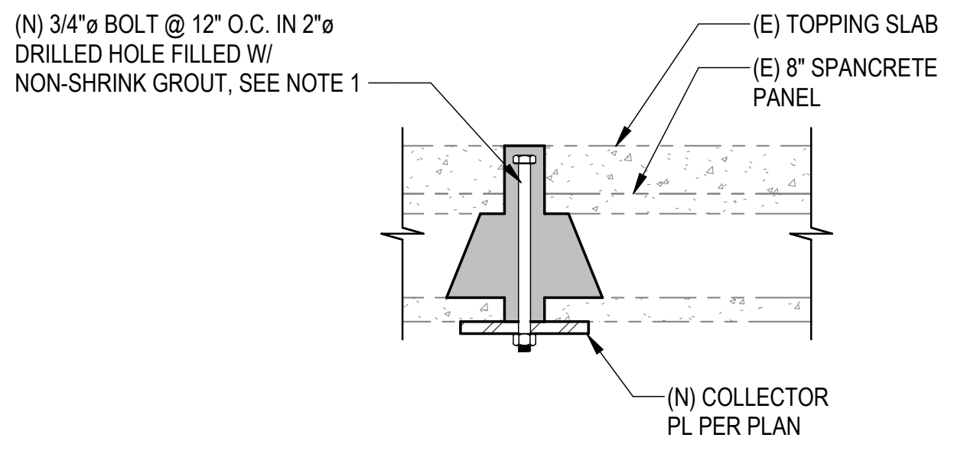
NOTES:
1. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

14 DIAPHRAGM-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.
2. LOCATE EPOXY DOWELS TO MISS REINFORCEMENT IN WALL.

10 COLLECTOR-TO-WALL CONN.
S201 SCALE: 1" = 1'-0"



NOTES:
1. DRILLED HOLES SHALL BE CENTERED ON VOID IN SPANCRETE PLANK AND LOCATED TO MISS PT TENDONS.

5 COLLECTOR SECTION
S201 SCALE: 1" = 1'-0"

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ISSUE DESCRIPTION

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	PLAN CHECK	01/15/2026

SHEET TITLE
DETAIL

SHEET NUMBER
S201